UNINTENDED CONSEQUENCES

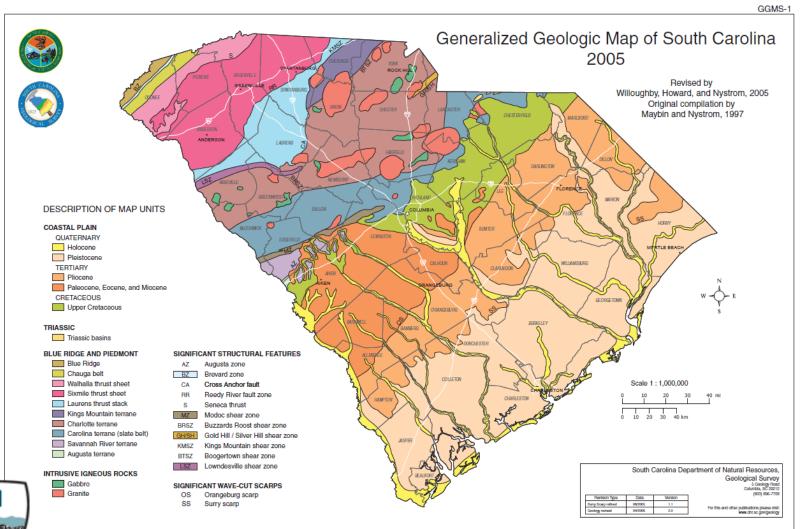
KARST IN SOUTH CAROLINA'S LOW COUNTRY

Stephen Geiger, P.E. ECS Southeast, LLP April 9th to 11th, 2018





SOUTH CAROLINA GEOLOGY

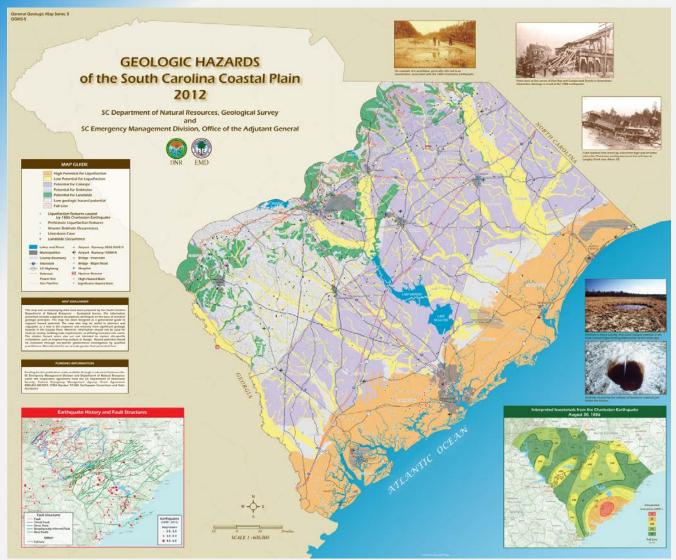




ASCE



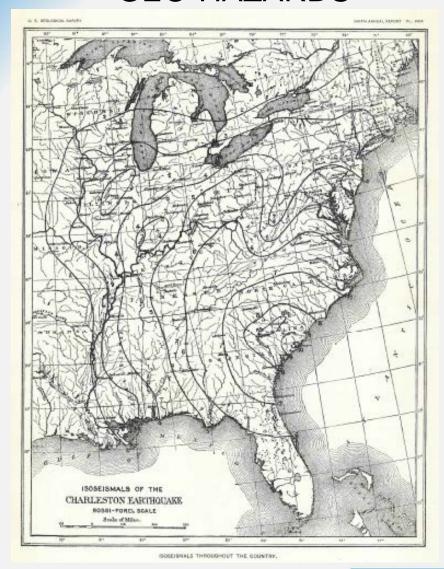
SOUTH CAROLINA COASTAL PLAIN GEO-HAZARDS







SOUTH CAROLINA COASTAL PLAIN GEO-HAZARDS







SOUTH CAROLINA COASTAL PLAIN GEO-HAZARDS IN AREA OF INTEREST

MAP GUIDE

- High Potential for Liquefaction *
 - Low Potential for Liquefaction
 - Potential for Collaspe
- Potential for Sinkholes*
- Potential for Landslide
 - Low geologic hazard potential
- Liquefaction features caused by 1886 Charleston Earthquake
- Prehistoric Liquefaction features *
- Known Sinkhole Occurrences *
- Limestone Cave
 - Landslide Occurrence

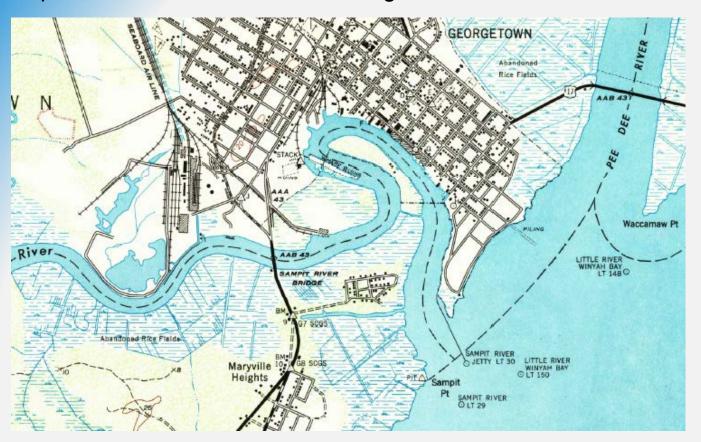




GEORGETOWN, SOUTH CAROLINA

Third Oldest City in the United States

Spaniards 1526, French 1562, English 1721, Chartered 1729





County Population = 63,000 MHI = \$41,500



GEORGETOWN'S "MOST" FAMOUS SON







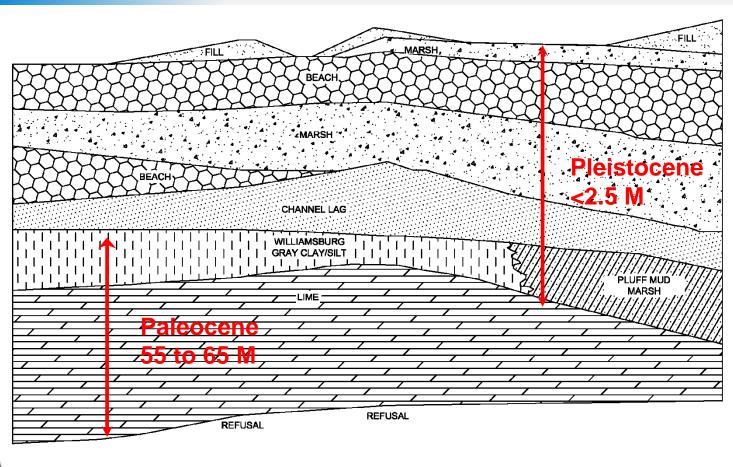
DOWNTOWN AREA OF INTEREST







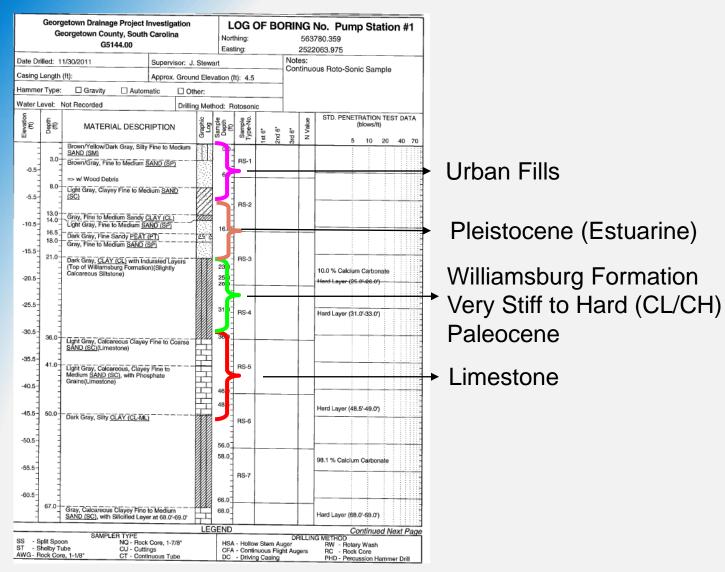
GEOLOGY SNAP SHOT







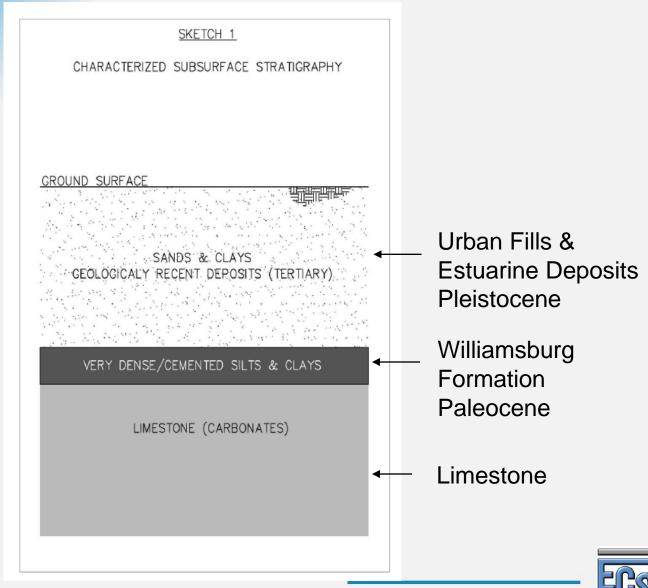
ROTOSONIC BORINGS





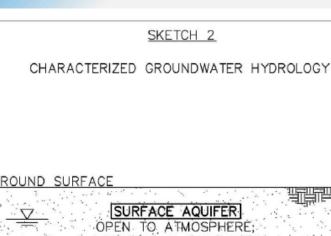


SIMPLIFIED GEOLOGIC PROFILE





SIMPLIFIED HYDRO-GEOLOGIC PROFILE



OCCURING IN SOILS WITH OW TO MODERATE HYDRAULI CONDUCTIVITY

AQUITARD - DOES NOT READILY TRANSMIT WATER;

∨ VERY LOW HYDRAULIC CONDUCTIVITY

LOWER CONFINED AQUIFER* -

NOT HOMOGENOUS OR ISOTROPIC. CONTAINS LIMESTONE DEPOSITS WITH HIGH VOID CONTENT AND SOLUTION CAVITIES.

VERY HIGH HYDRAULIC CONDUCTIVITY

*THE PRE-CONSTRUCTION HEAD PRESSURE IN THIS LOWER CONFINED AQUIFER IS EQUAL TO OR SLIGHTLY HIGHER THAN THE SURFACE AQUIFER

Discontinuous

High Void Content

Solution Cavities

Very High Hydraulic Conductivity



STORMWATER DRAINAGE PROJECT

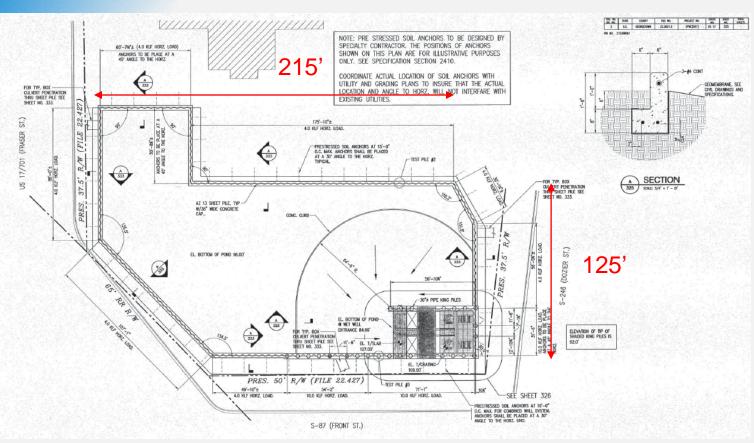




Significant Flooding During Heavy Rainfall and High Tide Confluence of 2 Major River Systems In Area Bounded by Blue Line



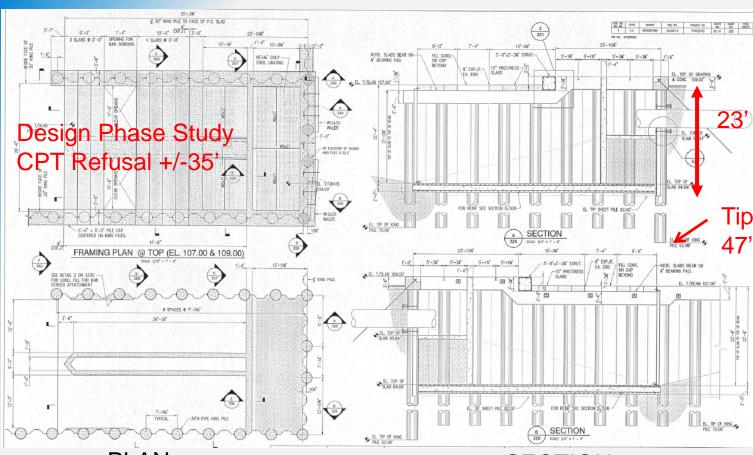
WET WELL CONFIGURATION





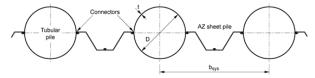


WET WELL DESIGN KING PILE SYSTEM



PLAN

SECTION



30" O.D. @ 7.25' O.C.

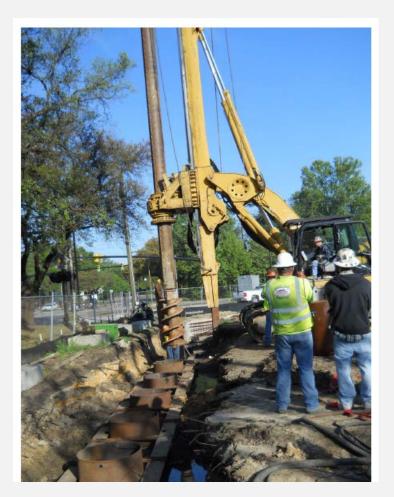




G-I







Temporary Casing Clean Out









Steel Shell Driven To Design Tip Elevation





G-I



Installed System
Prior to Concreting and Excavation



Concrete Placement Tremie Method







Artesian Conditions At Base of Wet Well Excavation



Deep Well

Williamsburg Formation

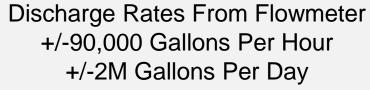




Finally A Dry Excavation!!



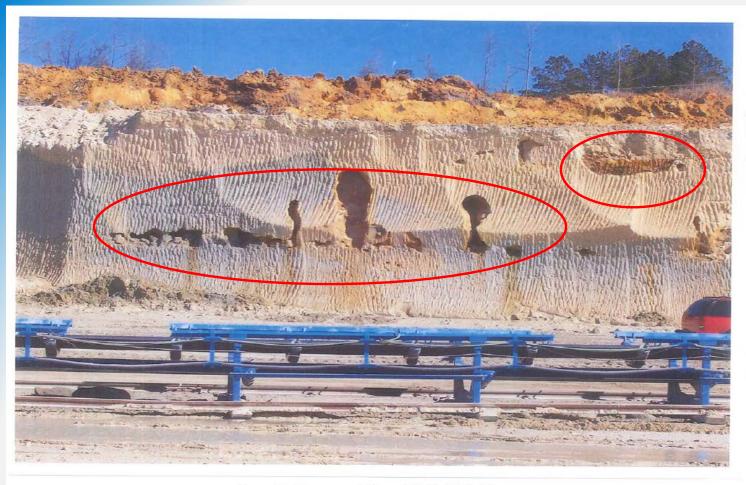








"REAL" GEOLOGIC CROSS SECTION







Open Cut @ Quarry < 20 Miles NW of Georgetown



HYDRO-GEOLOGIC CONSIDERATIONS

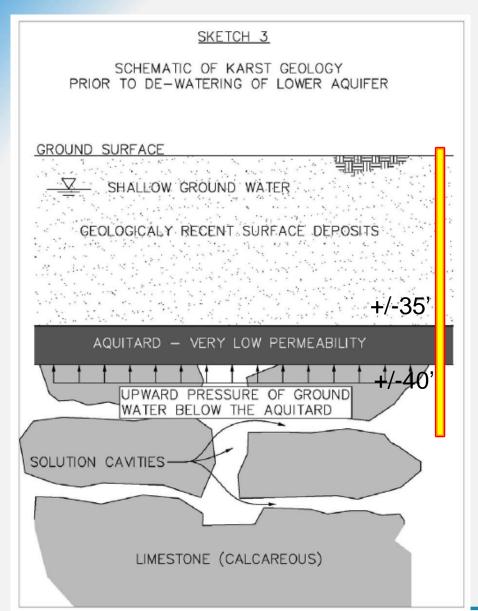
Shallow Dewatering

March/October 2011

Deep Dewatering

October/November 2011



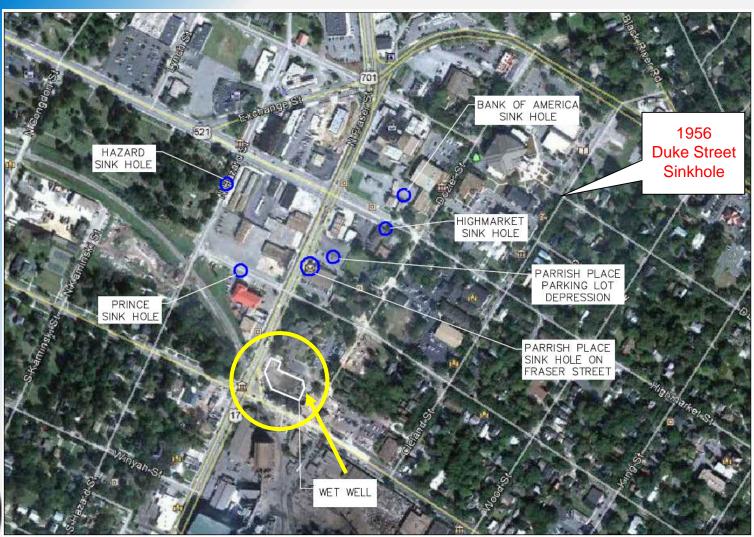


King Piles +/-47'

Conditions Prior To Wet Well Dewatering



DEWATERING EFFECTS

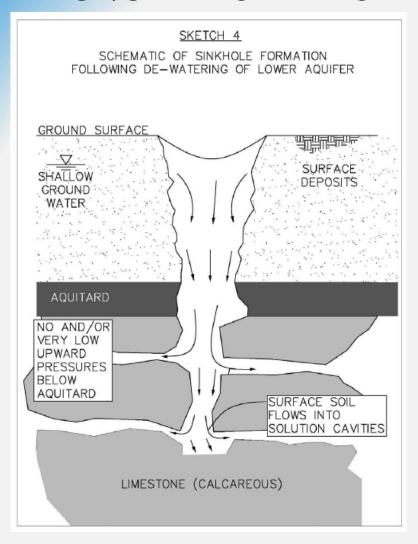






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COLLAPSE/SINK HOLE MECHANISM



Phenomena Documented In Coastal South Carolina Since 1975

Jamestown Quarry +/-35M Gallons Per Day – Widespread Subsidence



PARRISH PLACE PARKING FEATURE

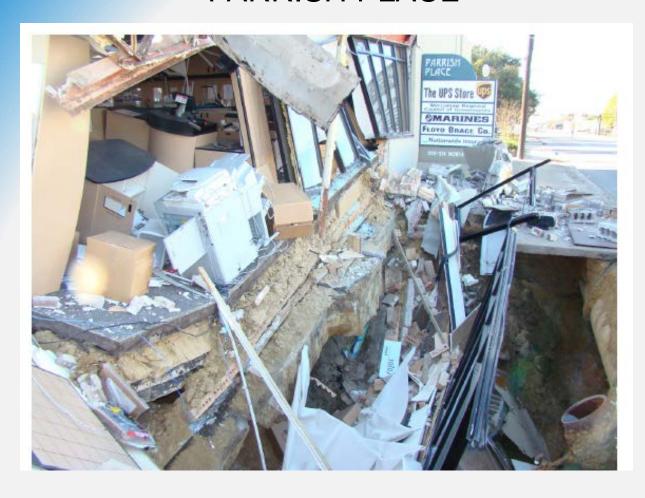




Opportunity for A Safety Minute?



UPS BUILDING COLLAPSE PARRISH PLACE



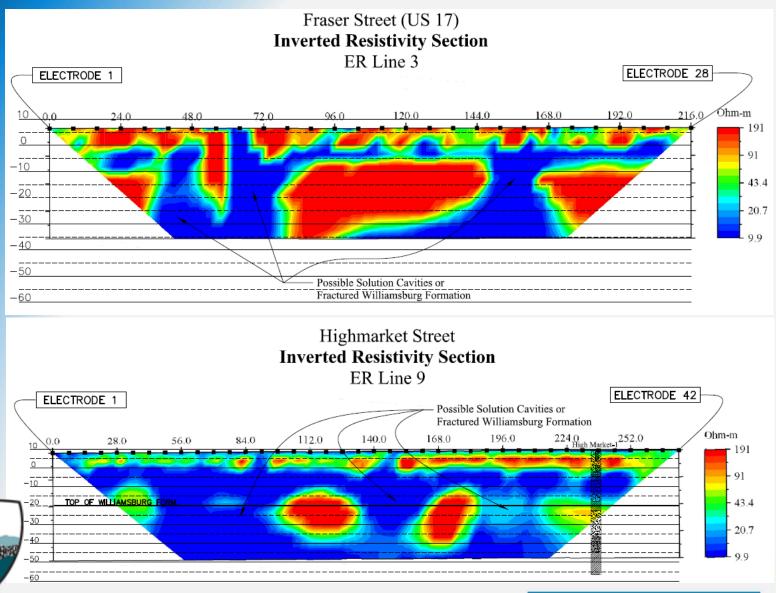


Collapse Occurred +/- 10pm November 17, 2011



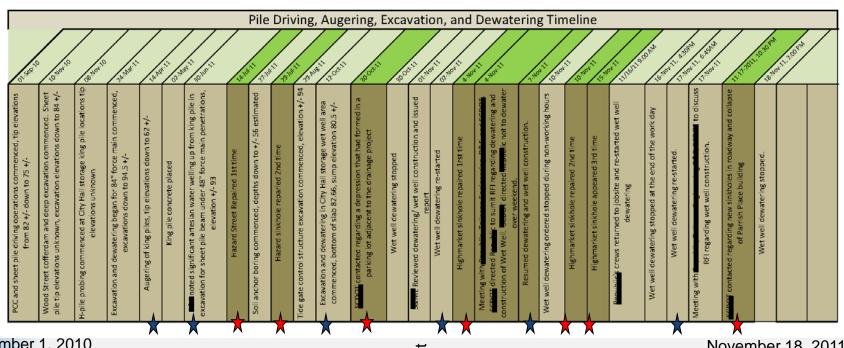
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GEOPHYSICAL CHARACTERIZATION





WET WELL PROJECT TIMELINE SIGNIFICANT EVENTS

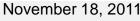


September 1, 2010

G-I

Conditions June 2011

Endorsement Consultant Nov. 2011





Consequence of Construction

★ Construction Activity of Note



THE "PROJECT"

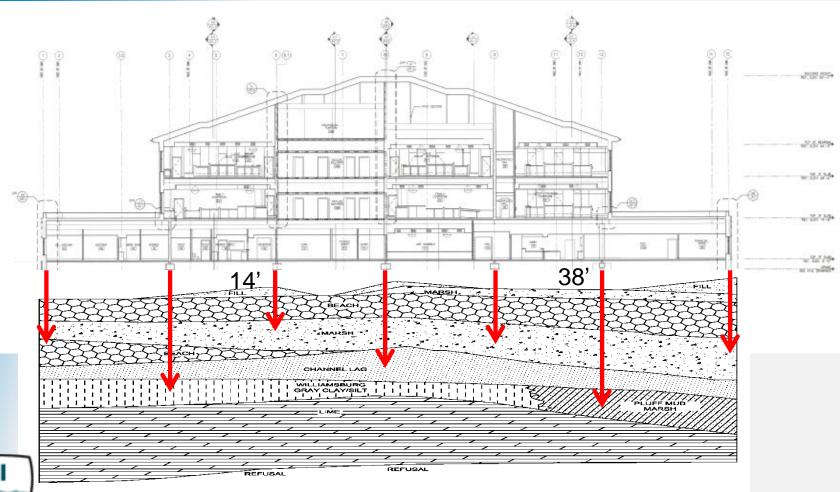




Steel and Masonry, 3-Story, 79,300 SF County Judicial Center Completed Spring 2008 Construction Value +/-\$18,000,000



THE "PROJECT"







GEOTECHNICAL DESIGN

Site formerly occupied multiple generations of structures

Grades raised approximately 4 to 6 feet

8" Tip Southern Pine Timber Piles** (VE)

35 feet minimum embedment

35 blows per foot at EOD

30 tons compression

10 tons uplift

4 tons lateral

Ground level concrete slab-on-grade ** (VE)

Geotechnical Exploration Methods? Geotechnical Testing Protocol? Conflicting Geotechnical Reports? For The Sake of \$?





GEOTECHNICAL CHALLENGES





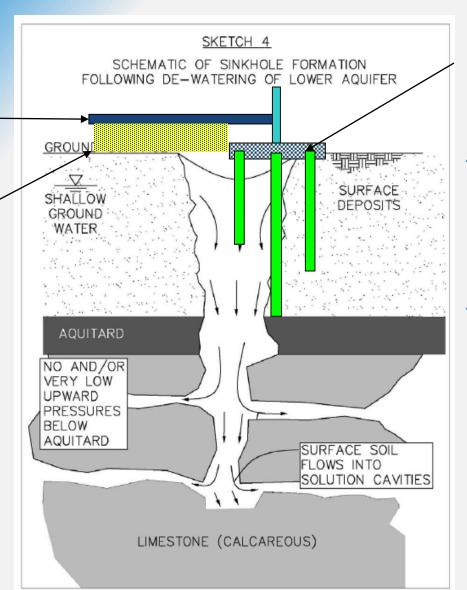
E.O.D. Resistances: 6 bpf to > 100 bpf



GEOTECHNICAL CHALLENGES

First Floor Slab On Grade

+/-4' to 6' New Fill



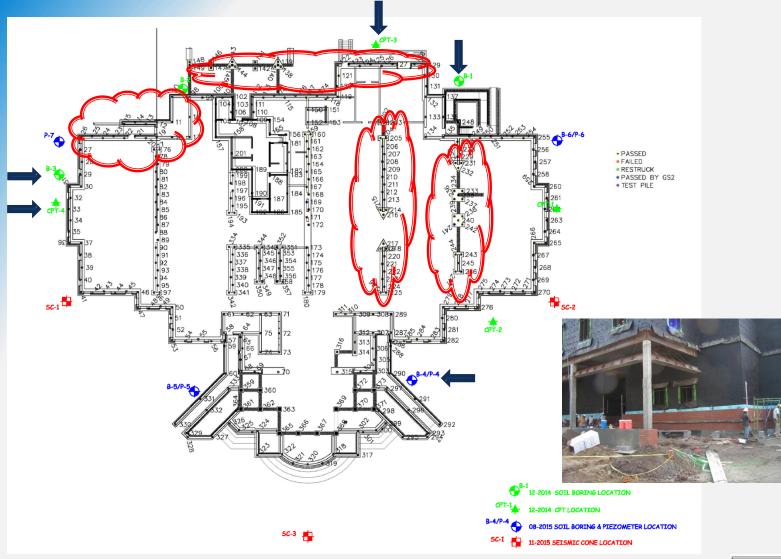
Pile Supported Load Bearing Elements

+/- 35' to 40'





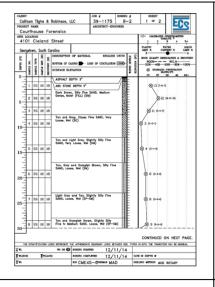
DISCOVERY AND CONSEQUENCE

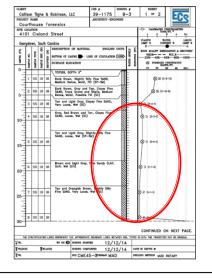






SUBSURFACE CONDITIONS POST WET-WELL



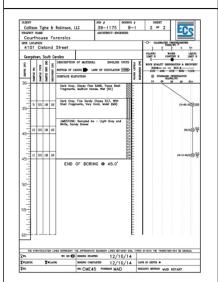


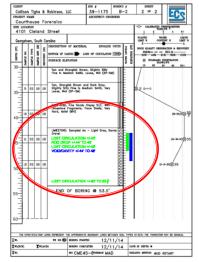
Poor Agreement Mud-Rotary Forensic Phase

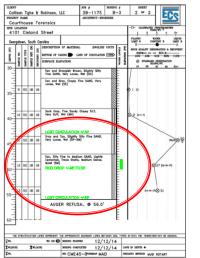
VS.

Hollow Stem Auger Design Phase



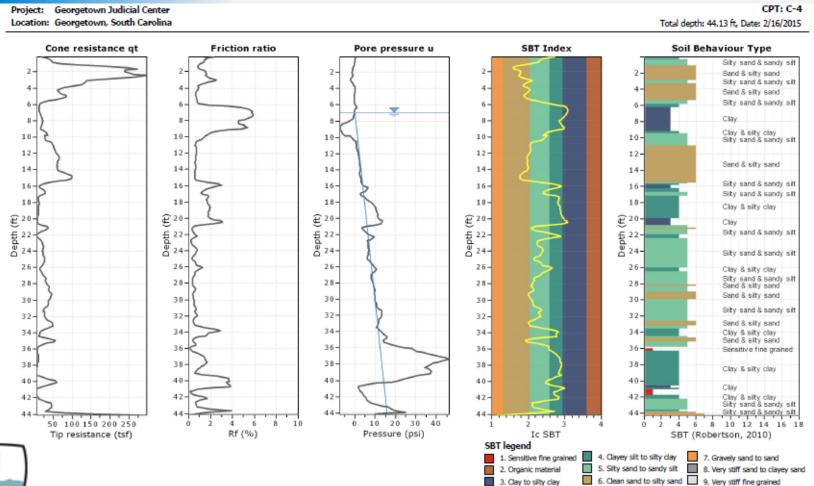








SUBSURFACE CONDITIONS POST WET-WELL







SUBSURFACE CONDITIONS POST WET-WALL

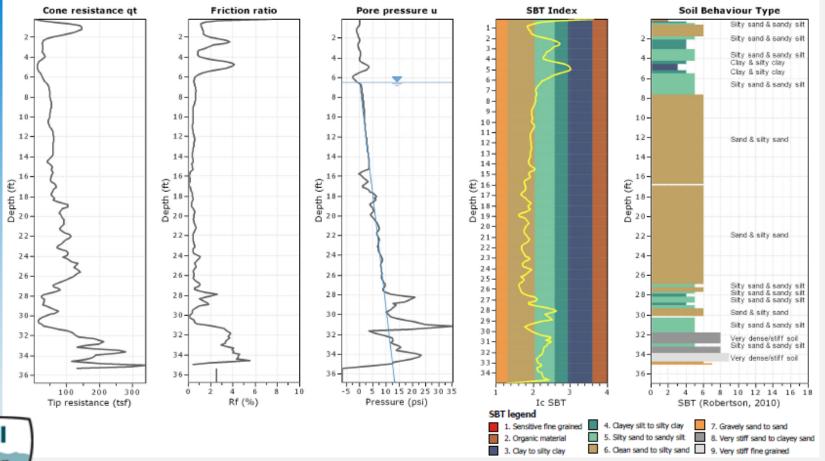
Project: Georgetown Judicial Center

Location: Georgetown, South Carolina

CPT: C-3

Location: Georgetown, South Carolina

Total depth: 36.78 ft, Date: 2/16/2015







WHERE ARE WE NOW?

Lawsuits Related To Drainage Project (>25)

Lawsuits Against Judicial Center Design Team

Protracted Litigation (> 6 Years)



Geotechnical and Structural Deficiencies

ECS Characterizes Geotechnical Deficiencies for Owner

ECS Undertakes Independent Analyses for Owner

Forensic Assessment w/REA

CPT & SCPT Soundings

Mud Rotary SPT Borings

Refraction Microtremor

SSHA

Liquefaction Triggering Analysis

Foundation and Slab Repair Design w/REA





WHERE ARE WE NOW?

Key Geotechnical and Foundation Design Shortcomings

Liquefiable Deposits Upper 35 feet ** (+/-2" to 5")

Peat & Very Soft Marine Clay Layers (LL's > 85, Pl's > 45, q_t < 5 tsf) **

Installed Pile Lengths 14 to 38 feet **

Working Pile Loads

Actual Compressive As Much As 25% Greater Than Design (REA)

Actual Uplift As Much As 20% Greater Than Design (REA)

Actual Lateral As Much As 75% Greater Than Design (REA)

Interpreted Pile Capacities As Low As 50% of Working Loads (FS=1)



Timber Piles Structurally Inadequate
Working Stresses During Lateral Loading >>>> Allowable



WHERE ARE WE NOW?

Conceptual Geotechnical Repairs

Foundations

Underpinning w/ Cased Micropiles (\$5M per HBI) **

Ground Level Slab

New Micropile Supported Structural Slab (\$2M per HBI) **

HDPE Grouting of Pleistocene Deposits (\$1M) X

Low Mobility Grouting of Limestone (\$2M to \$3M) X

Collateral Work

Remove and Replace Existing Finishes (\$3M to \$4M)



Definition of "Sinkhole" – Engineering vs. Coverage

County Operations Relocation During Repairs (18 mos. @ \$6M to \$8M)

Estimated Damages (\$15M to \$18M or more)



G-I

FUN FACTS

Originally Considered Driven 55' PSC or ACIP & Structural Floor Slab Value Engineered - Driven Timber Piles and Slab On Grade (\$600,000 net)

Hollow Stem Auger Borings by GER (Max. Depth 35')

No Laboratory Testing by GER

Conflicting Versions of GER Design Level Reports (seismic risks????)

Design Level Geotechnical Reports Don't Bear Firm or EOR Seals

+/-375 Timber Piles Installed September/October 2007

Production Pile Lengths 14 to 38 Feet

2 Pre-Production Static Load Tests w/Questionable Results

6 of 10 PDA Tests During Driving, Axial Capacity Concerns

GER Not Retained For Construction Observations (would it have mattered?)

CMT/SI Firm Refuses To Write Letter "Certifying" Foundations

GER Reviews SI Driving Records, "Certifies" Pile Capacity (FS = 1.5)

THANK YOU!









Shameless Plug!!!!

