

Lessons learned from Ground Improvement projects Around the World

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Around the world,
Geotechnical Engineers
face many of the same
challenges

Principle 1 :
Owners don't want to pay
for a thorough
geotechnical investigation
incl. Lab testing -> There's
never enough
information



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Principle 2 : Terzaghi is our god

$$\sigma = \sigma' + u$$

SOIL MECHANICS
IN ENGINEERING
PRACTICE

THIRD EDITION

Karl Terzaghi
Ralph B. Peck
Gholamreza Mesri

Principle 3 :

- The three main issues that may lead to the use of deep foundations or ground improvement are :

SETTLEMENT /
HEAVE

BEARING
CAPACITY

LIQUEFACTION



To deal with these three issues, there are basically three ways :

CONSOLIDATE



Vacuum consolidation



Vertical drains

DENSIFY



Rapid impact compaction



Dynamic compaction



Vibroflotation

STRENGTHEN



Deep soil mixing



Rigid inclusions



Stone columns



Bi-modulus columns



Dynamic replacement

WICK DRAINS

- Band-Shaped plastic strip – Accelerate consolidation of compressible soils



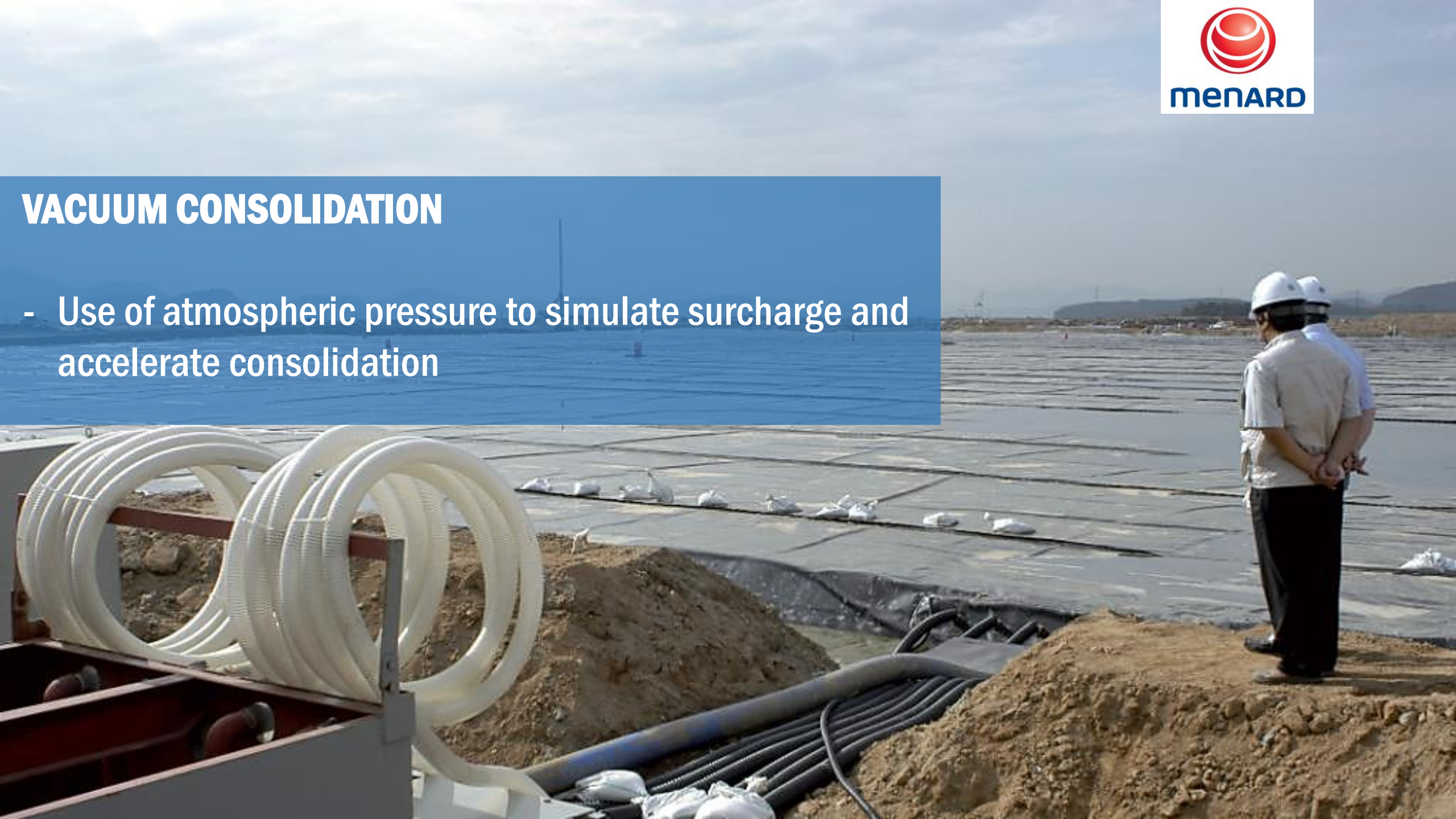


The Art of Wick Cutting with Jonah

US Wick Drain
Charleston 2016

VACUUM CONSOLIDATION

- Use of atmospheric pressure to simulate surcharge and accelerate consolidation



DYNAMIC COMPACTION

- Free fall of 12-20 tons weights from 50-100 ft for compaction of granular soils



RAPID IMPACT COMPACTION (RIC)

- High Frequency tamping using a 9-12 tons weight





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A close-up photograph of a yellow safety vest. The vest features a black logo at the top left, consisting of three curved lines. Below the logo, the word "ENARD" is printed in large, bold, black capital letters. The bottom edge of the vest has a black and yellow diagonal striped pattern. The background is blurred, showing an indoor setting with a white counter and some items on it.



VIBROFLOTTATION

- High energy vibratory probe using water jets to densify clean sands







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CONTROLLED MODULUS COLUMNS (CMC)

- Grouted Rigid Inclusions installed with a displacement tool





STONE COLUMNS / AGGREGATE PIERS

Columns of Vibrated Compacted Crushed Stone –
Seismic mitigation



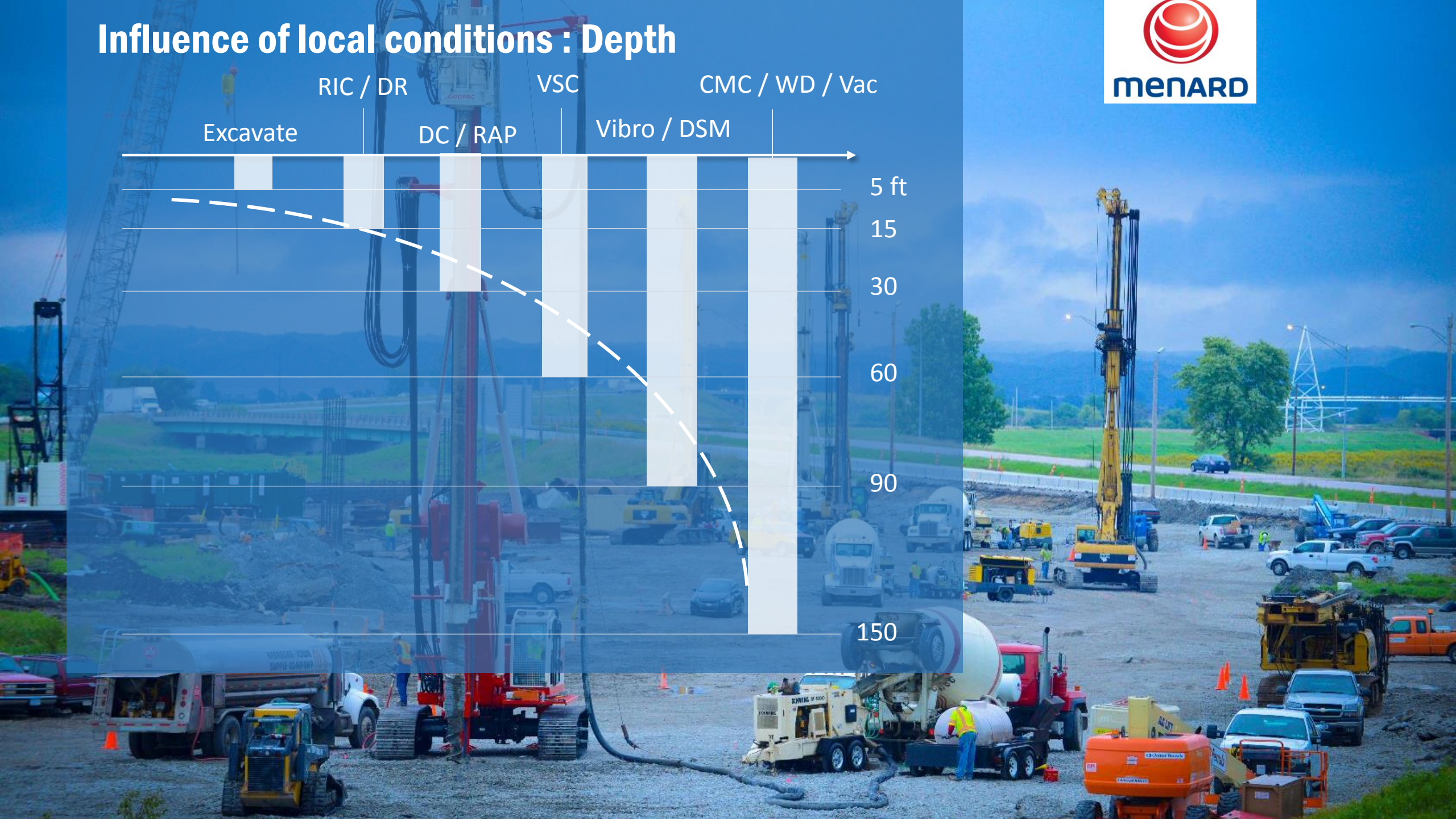
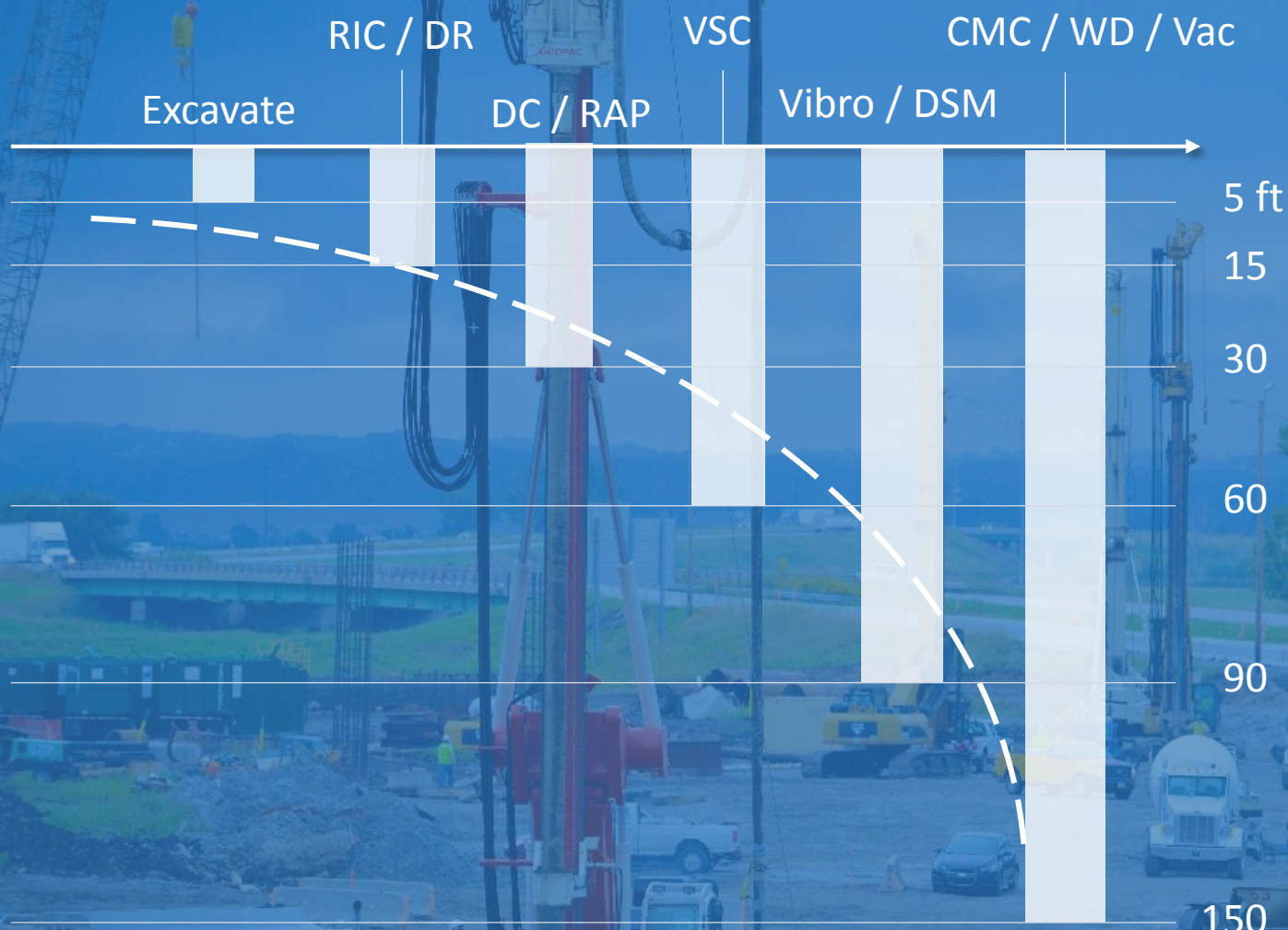


BI-MODULUS COLUMNS

- Combination of a Stone Column Installed Directly Above a Rigid Inclusion



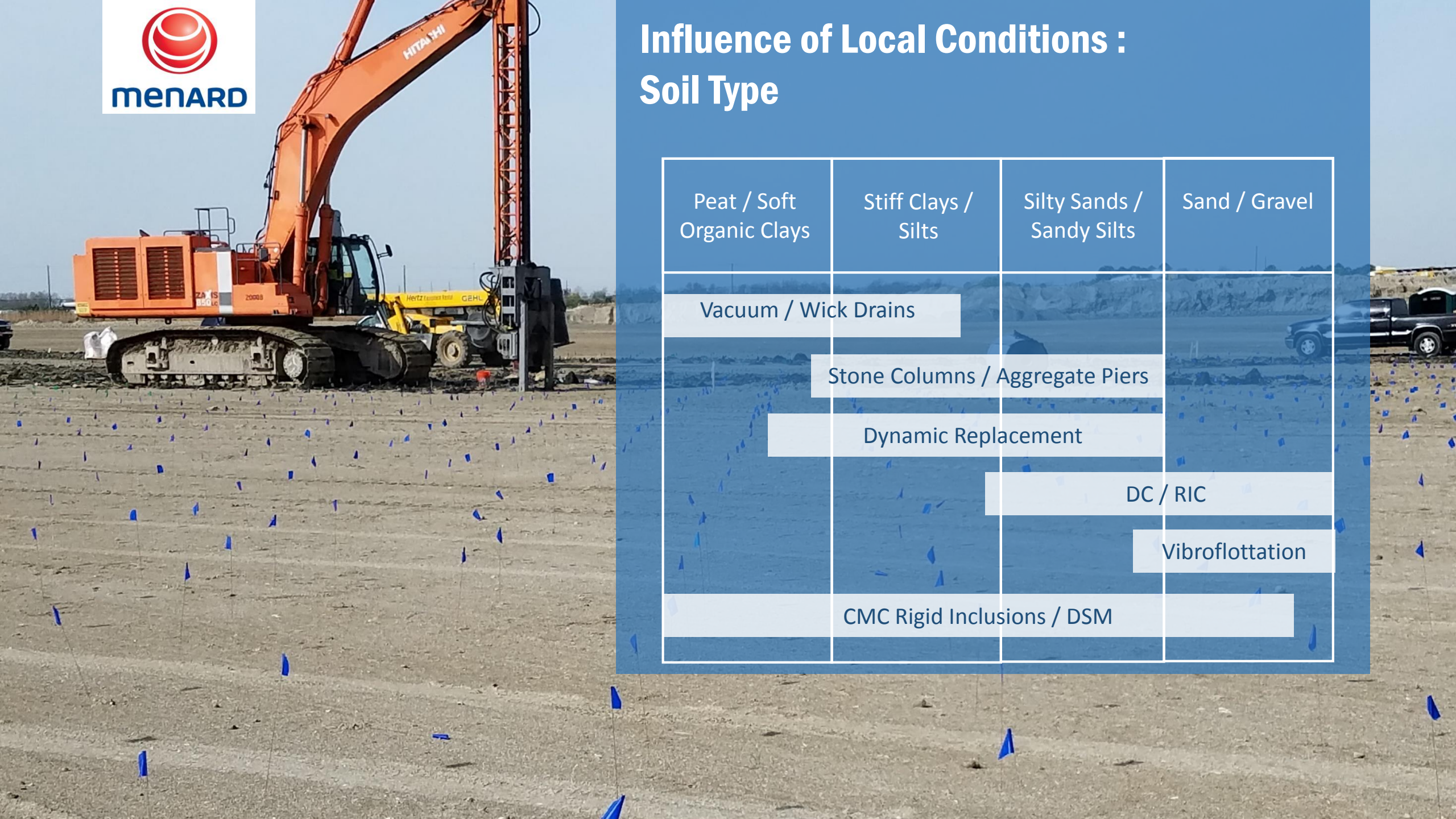
Influence of local conditions : Depth





Influence of Local Conditions : Soil Type

| Peat / Soft Organic Clays | Stiff Clays / Silts | Silty Sands / Sandy Silts | Sand / Gravel |
|---------------------------|---------------------------------|---------------------------|---------------|
| Vacuum / Wick Drains | | | |
| | Stone Columns / Aggregate Piers | | |
| | Dynamic Replacement | | |
| | | DC / RIC | |
| | | Vibroflotation | |
| | CMC Rigid Inclusions / DSM | | |





Project : Kimhae & Jangyoo Sewage Treatment Plant

Country : South Korea (Busan)

Summary : New plant in Greenfield with final grade raised several meters and large net new loads

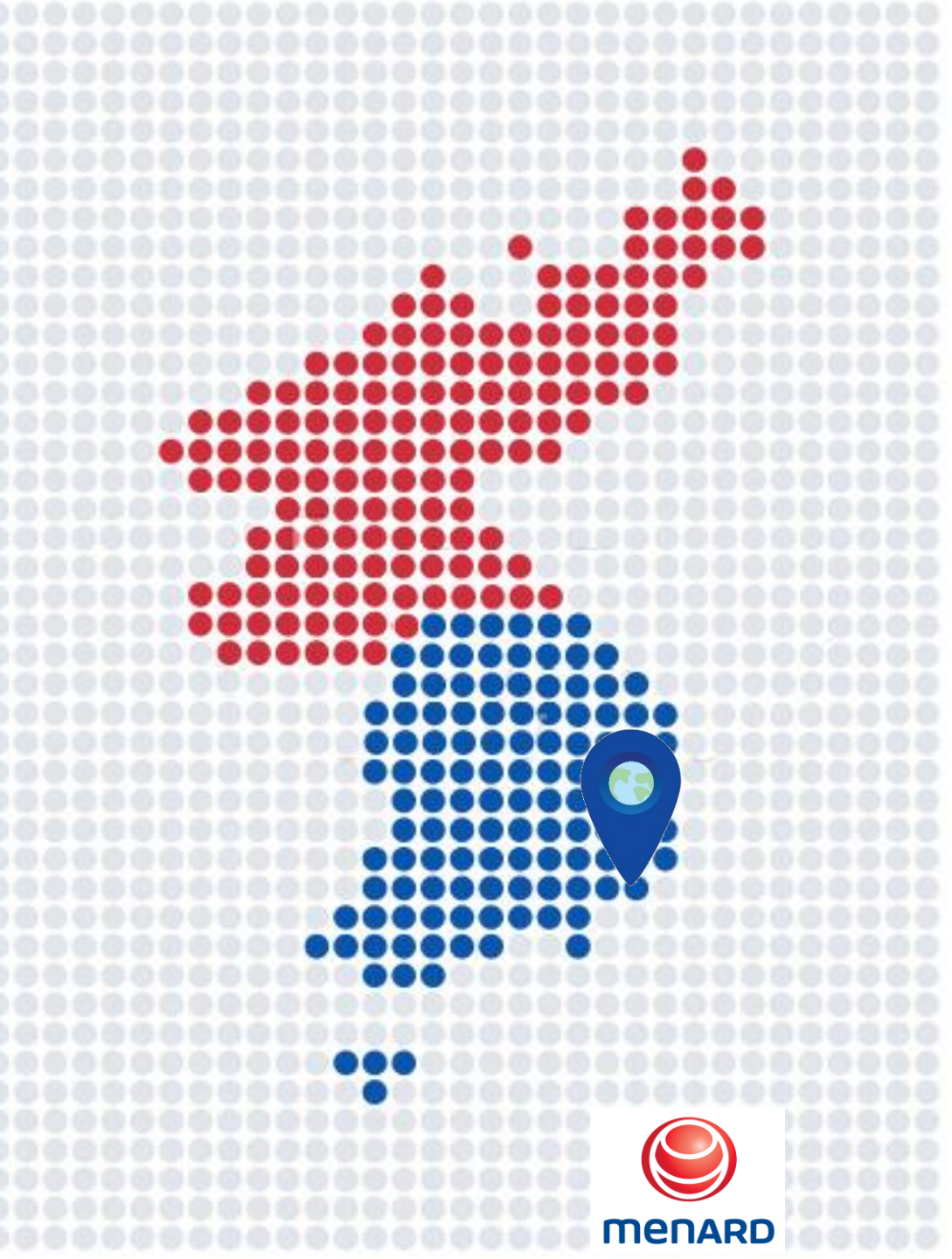
Main Issues to Solve :

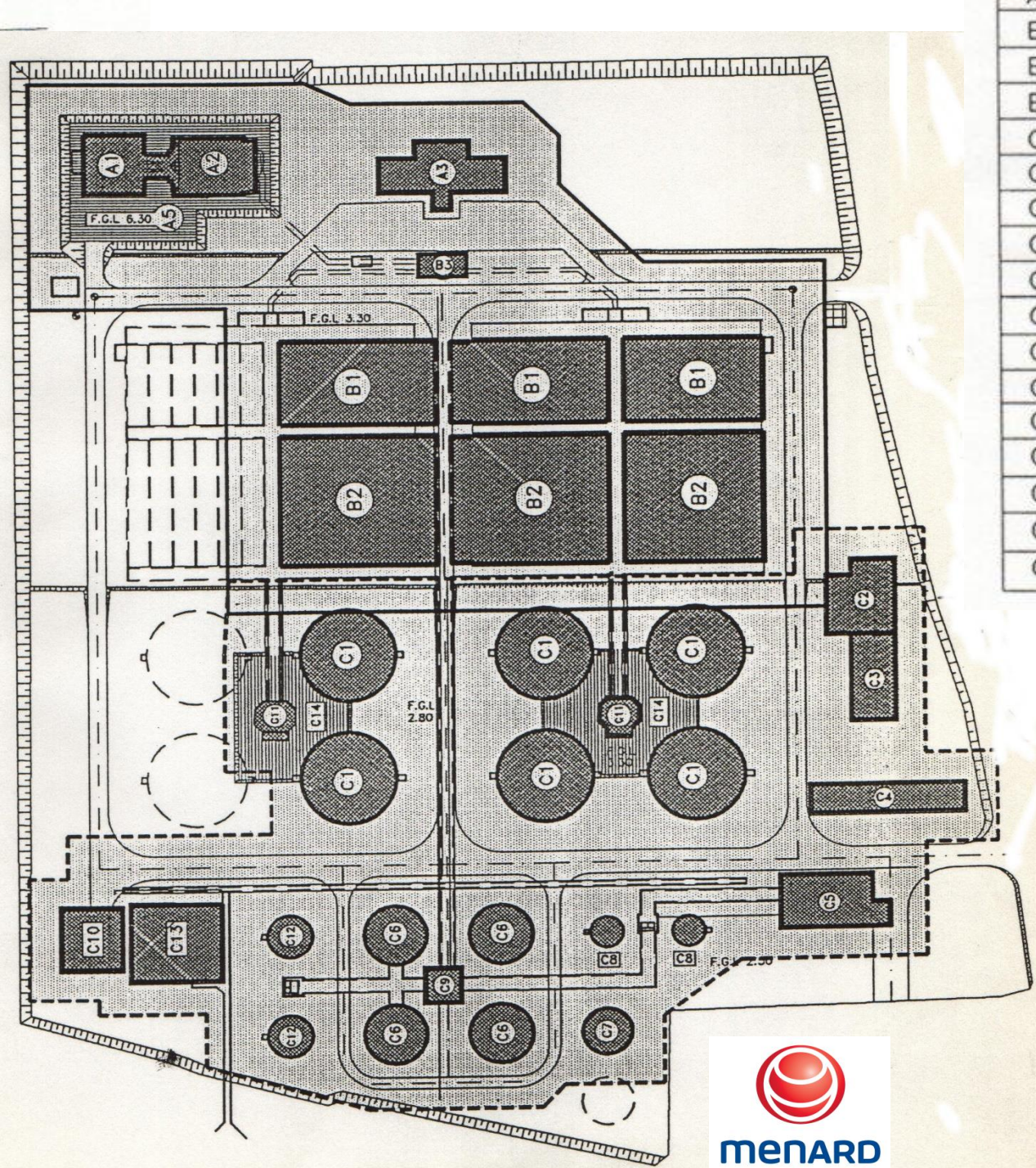
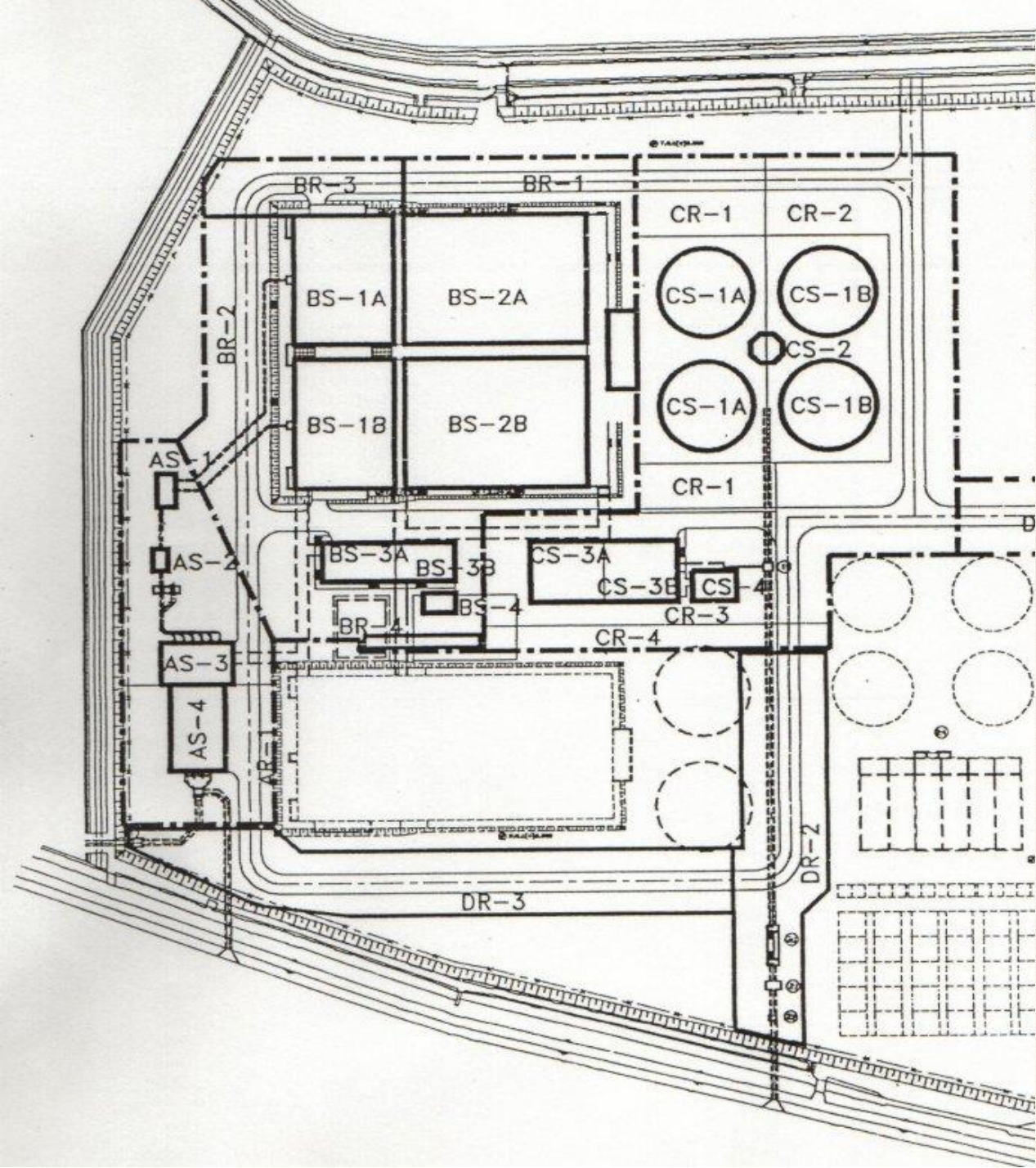
- Local condition : deep very soft young deposits (>45M) from NakDong River Valley
- Settlement : predicted long term settlement > 6M over 20 years due to deep soft clays
- Construction period – Fast schedule
- Lack of availability of fill material





Kimhae & Jangyoo Sewage Treatment Plant





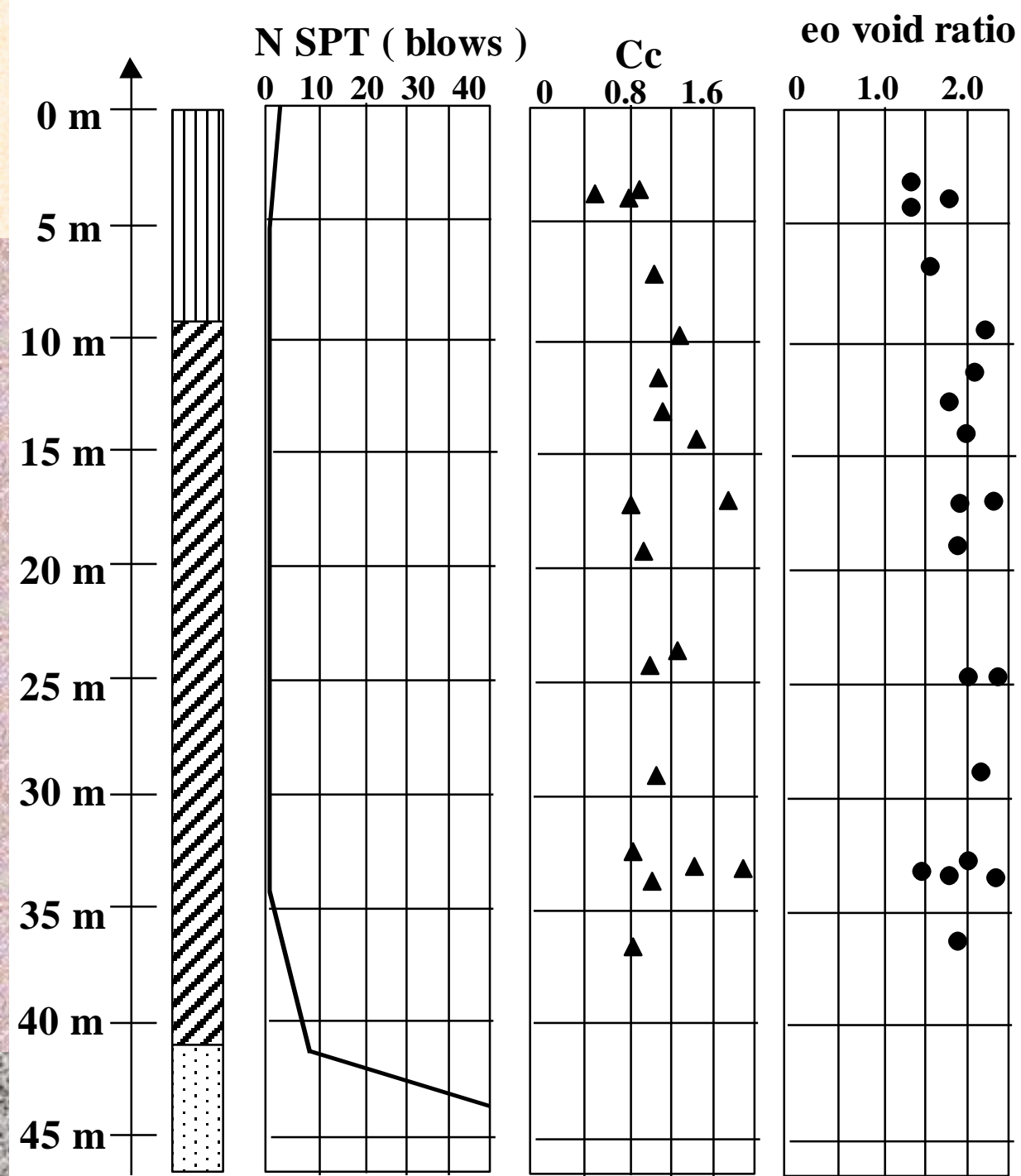


GENERAL DATA ON PROJECT :

- **160,000 m² (1.7M ft²)**
- **Construction Period : 1995-2000**
- **2 Water Sewage Treatment plant to meet the growing demand of boom of population of Kimhae and Jangyoo township, suburbs of Busan, 2nd largest city in South Korea**



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Silty Sand to Sandy Silt – 5m (17 ft)

Soft Organic clay– 40m (130 ft)

Weathered Rock



Silty Sand to Sandy Silt – 5m (17 ft)

Soft Organic Clay– 40m (130 ft)

Weathered Rock

Main Characteristics of the Soft Clay Layer :

normally Consolidated

Water content average 75%

$N(\text{SPT}) = 0$ to 1

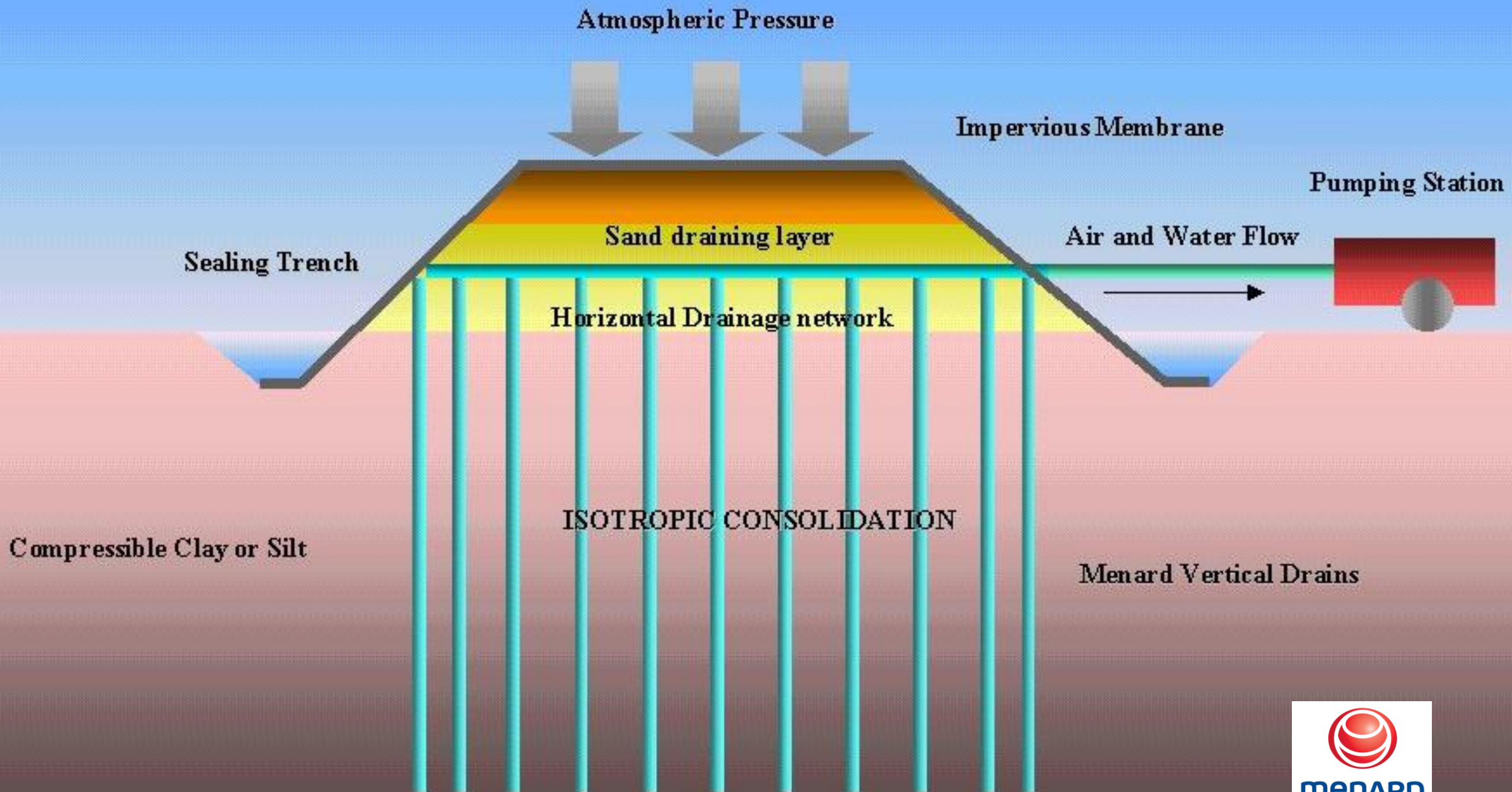
$C_c = 1.21$

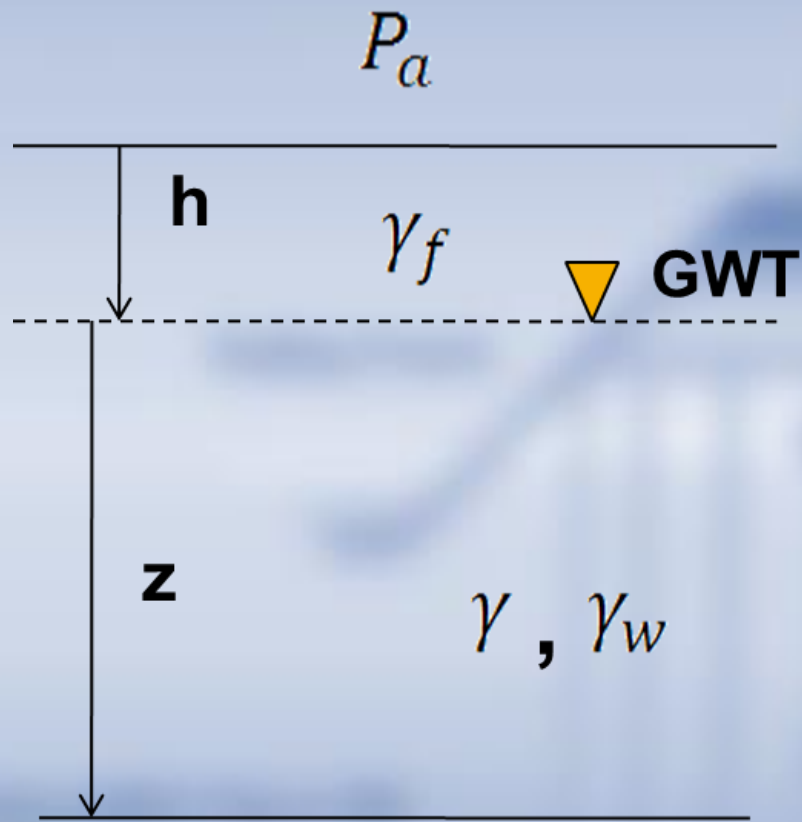
$e_o = 2.012$

$C_v = 1.32 \text{ m}^2/\text{y}$

Thickness between 25 and 40m (85ft to 130 ft)

Expected max settlement $> 6\text{m}$ (15% of clay thickness)





BEFORE VACUUM APPLICATION:

$$\sigma_T = \gamma z + \gamma_f h + P_a = \sigma_t + P_a$$

$$u_T = \gamma_w z + P_a = u_t + P_a$$

$$\begin{aligned} \sigma'_i &= \sigma_T - u_T = \sigma_t - u_t \\ &= \gamma' z + \gamma_f h \end{aligned}$$

AFTER VACUUM APPLICATION:

$$\sigma_T = \gamma z + \gamma_f h + P_a = \sigma_t + P_a$$

$$u_T = \gamma_w z + P_a - P_a$$

$$\begin{aligned} \sigma'_f &= \sigma_T - u_T \\ &= \sigma'_i + P_a \end{aligned}$$

$$\Delta\sigma' = nP_a \rightarrow \text{Where } n \approx 0.7 - 0.8$$



















START OF VACUUM / DEPRESSURE

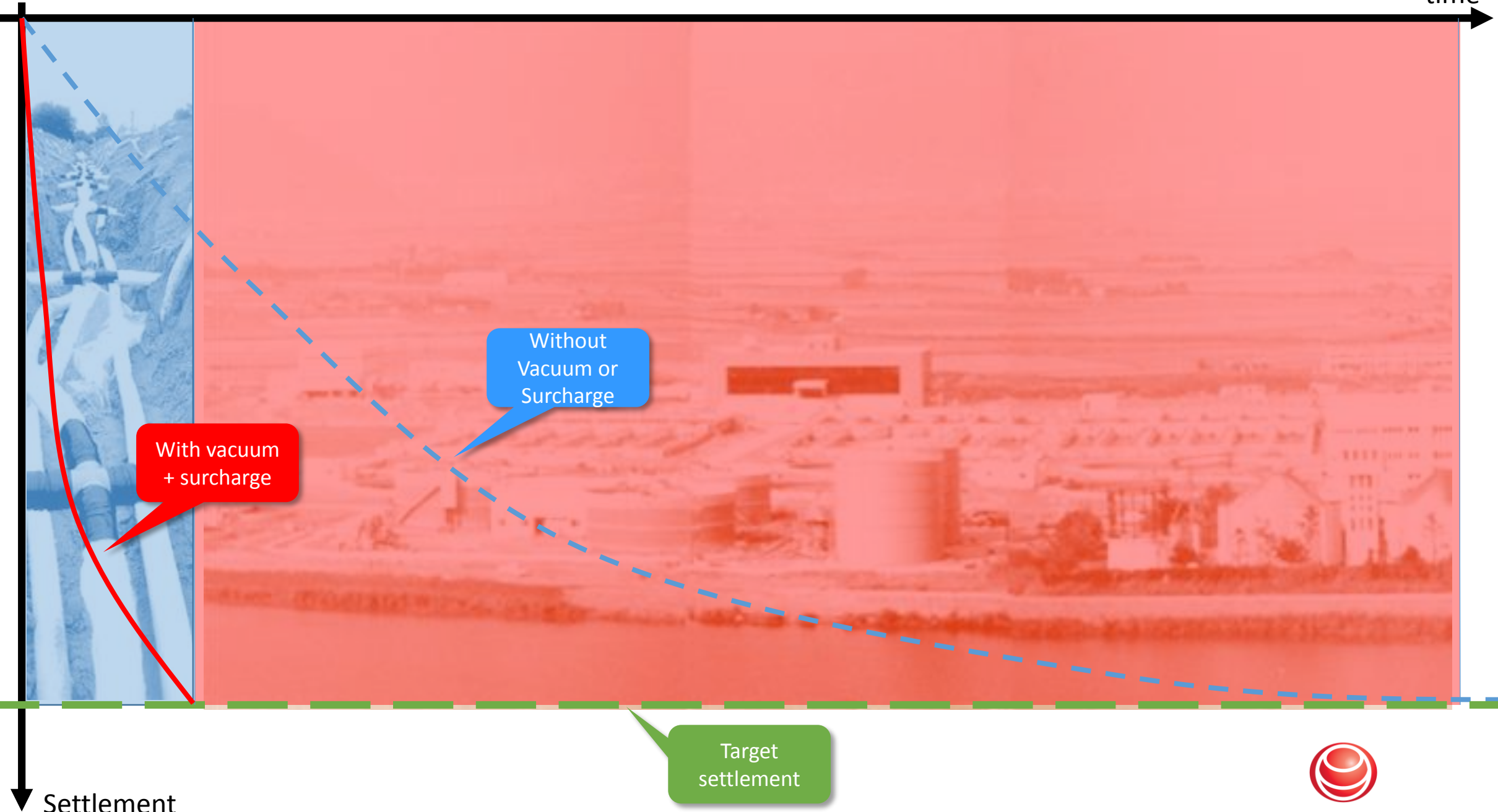




Construction period

Life of the structure

time



With vacuum + surcharge

Without Vacuum or Surcharge

Target settlement



• SETTLEMENT ANALYSIS & VACUUM STOP DECISION PROCESS

1D consolidation theory

$$\Delta\sigma'(z) = \gamma H_{fill} + \sigma_o + \gamma' \Delta H_{primarysettlement}$$

$$\Delta H_{primarysettlement} = \frac{C_c}{1+e_o} \cdot H \cdot \log\left(\frac{\sigma_o + \Delta\sigma'(z)}{\sigma_o}\right)$$

Calibration Coefficient

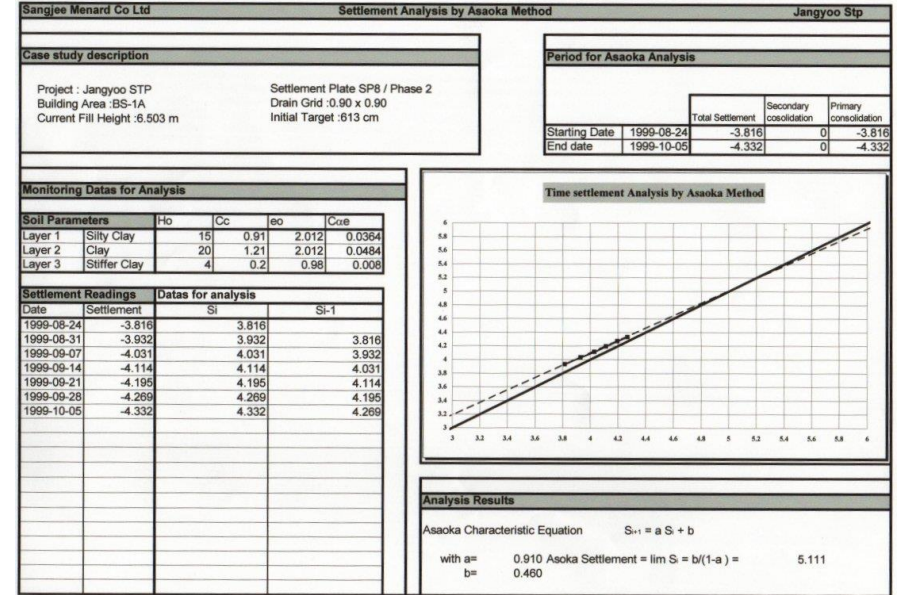
$$\beta = \frac{\Delta H_{asaoka}}{\Delta H_{theory}} = \frac{\left(\frac{C_c}{1+e_o}\right)_{actual}}{\left(\frac{C_c}{1+e_o}\right)_{soilinvestigation}}$$



Specifications



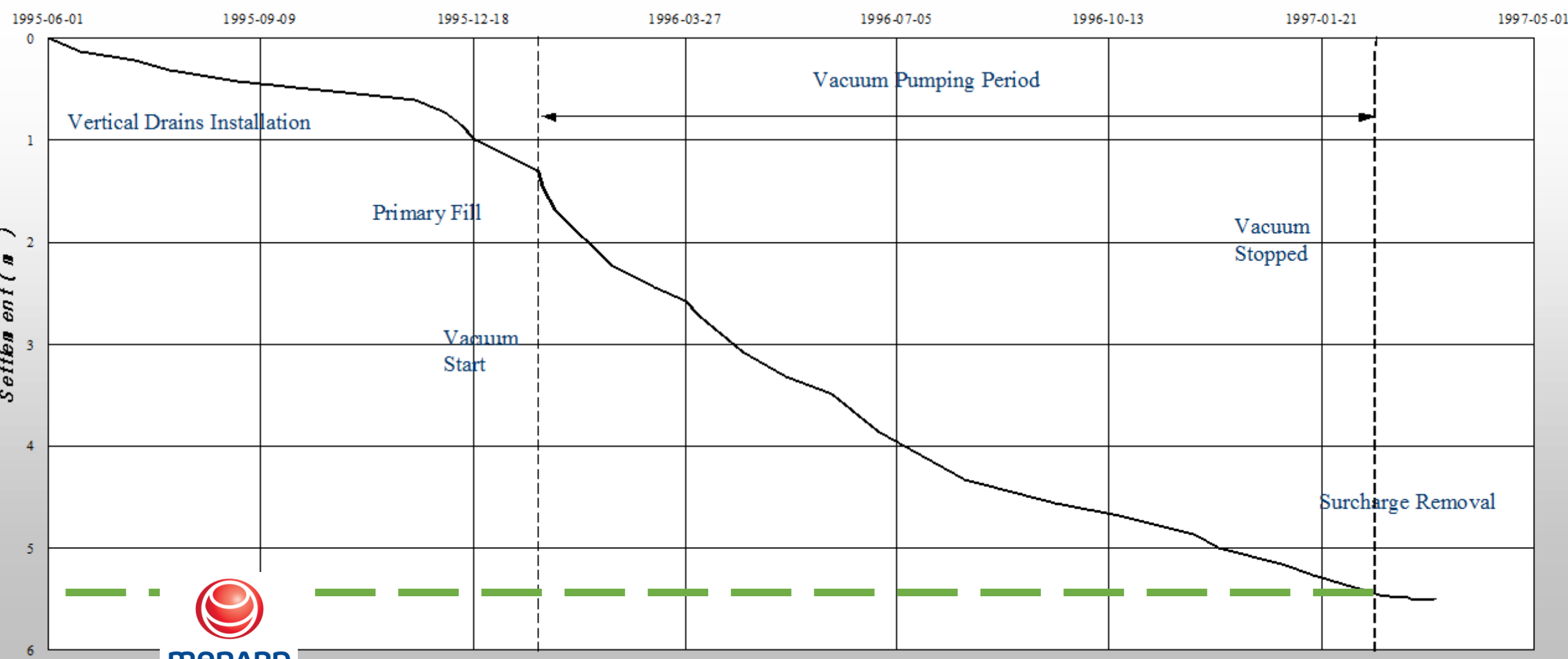
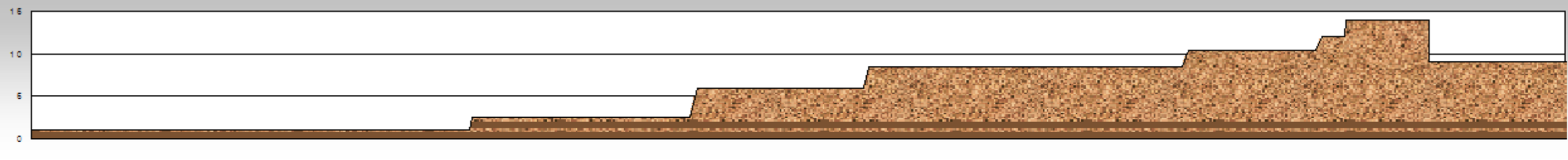
Asaoka Analysis of monitoring results



SETTLEMENT TARGET

Target reached ? Yes -> Stop vacuum

Target reached ? No -> Continue analysis







Project : Airbus A380 Assembly Plant

Country : Germany (Hamburg)

Summary : New Assembly plant on Elbe River for Airbus A380 – extension of runway

Main Issues to Solve :

- Local condition : underconsolidated river tidal deposits (Elbe River
- Settlement : predicted long term settlement > 2M over 20 years due to very compressible deposits (Muck)
- Bearing capacity / Slope Stability at edge







Hamburg -
Blankenese

ELBE

extension

Hahnöfer Sand

AIRBUS-
extension

Mühlenberger
Loch

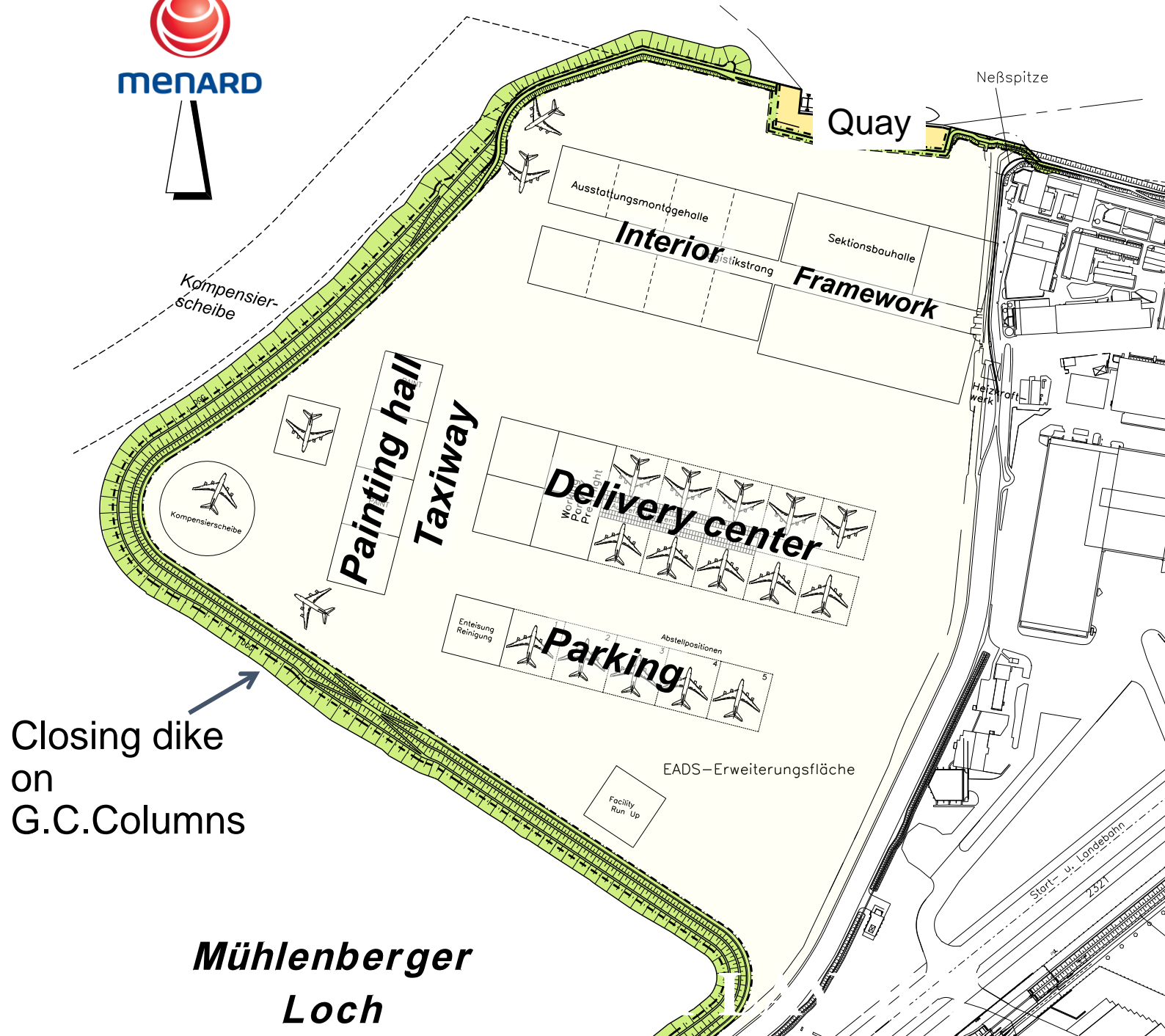
Jork

Hamburg-
Finkenwerder

Alte Süderell





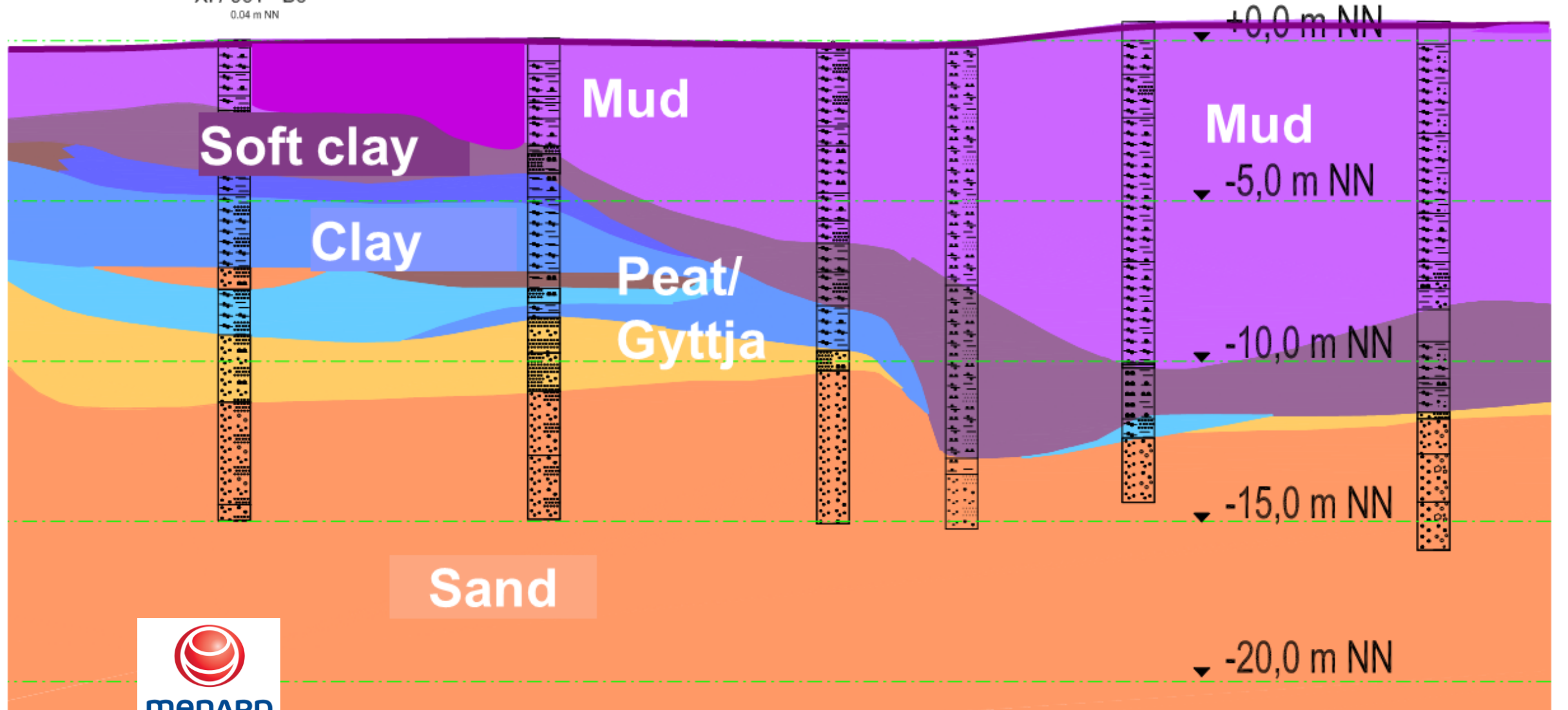


- Reclaimed Area = 170 ha (about 1.9m ft2)
- Final Assembly of Airbus A380
- Spare parts delivered by barge, plane or road to the Hamburg plant
- Containment dike on GCC
- Hydraulic sand reclamation
- Wick drains + Vaccum

VIIa

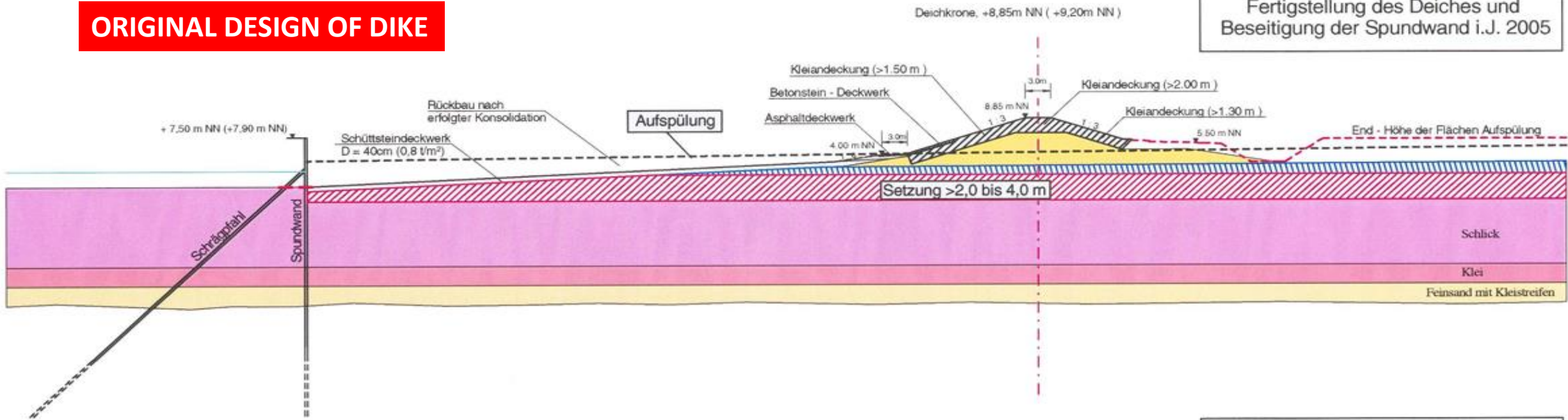
VII

XI / 961 - B6
0.04 m NN



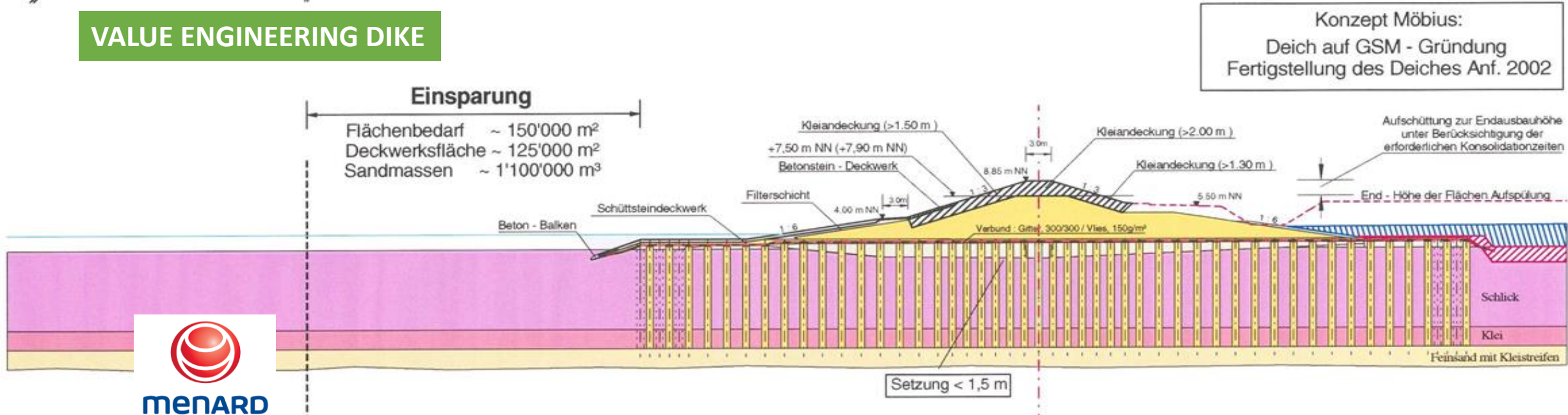


ORIGINAL DESIGN OF DIKE



Konzept der Ausschreibung:
Deich im Schutz einer temporären Spundwand
Fertigstellung des Deiches und Beseitigung der Spundwand i.J. 2005

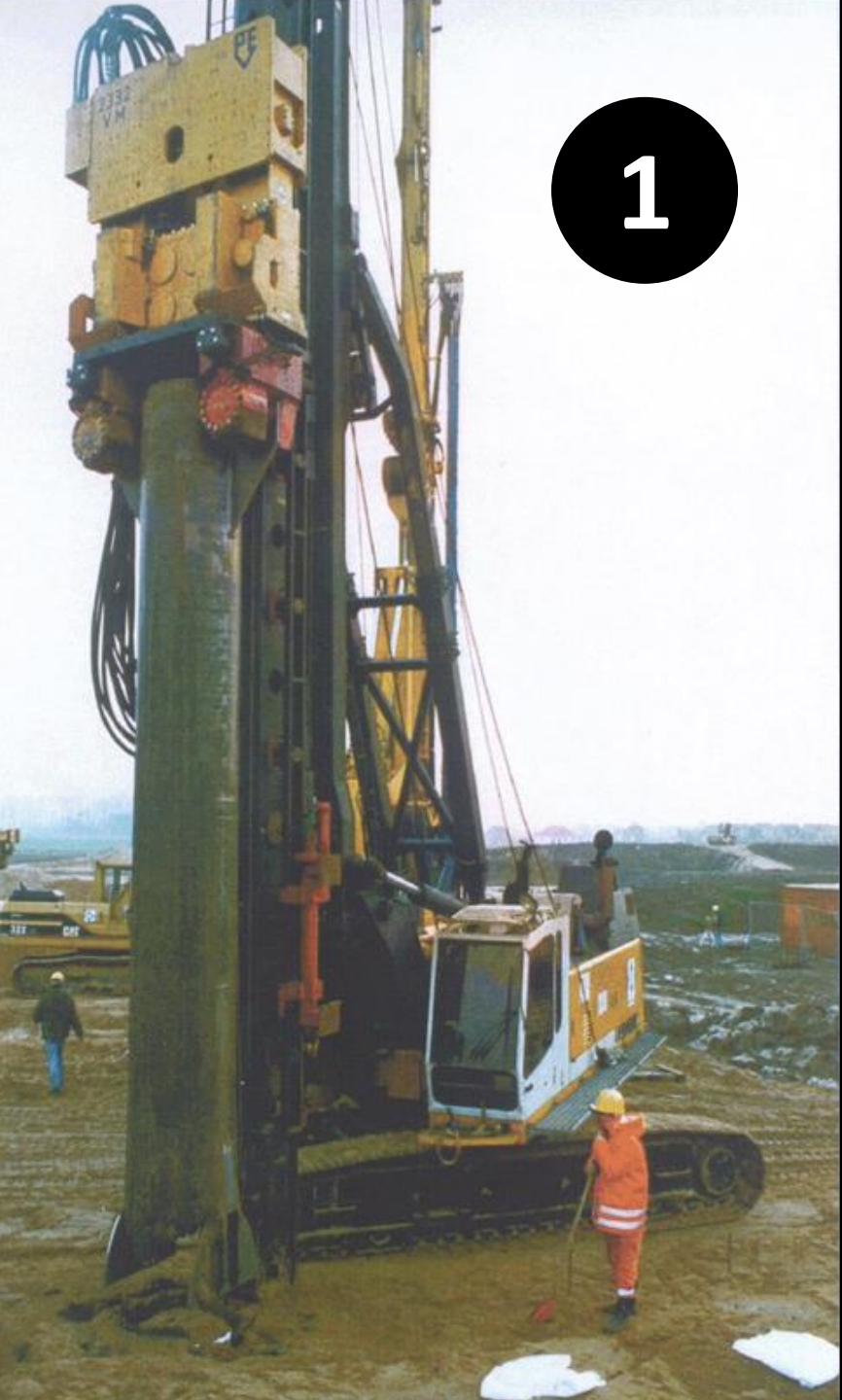
VALUE ENGINEERING DIKE



Konzept Möbius:
Deich auf GSM - Gründung
Fertigstellung des Deiches Anf. 2002



1



2



3

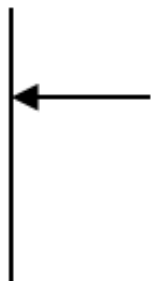












Site Limits

Dyke

Flooding of site
NN+3m

Sand spray

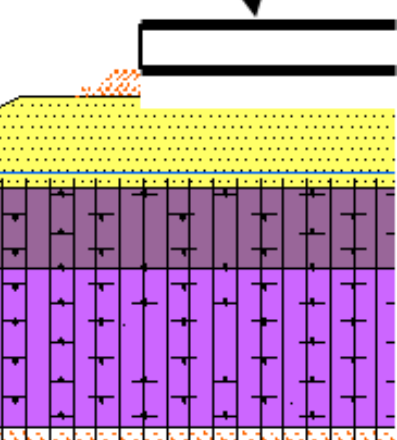
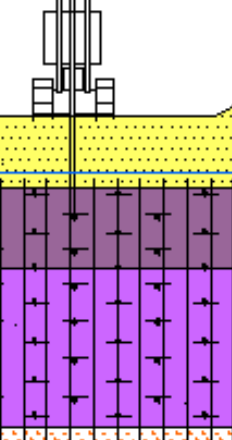
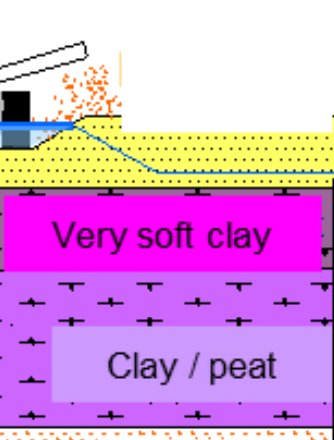
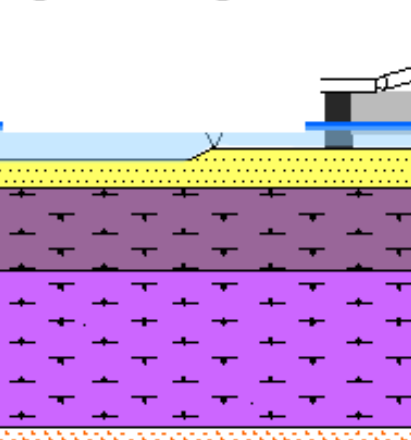
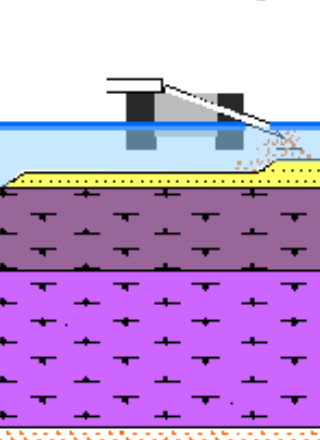
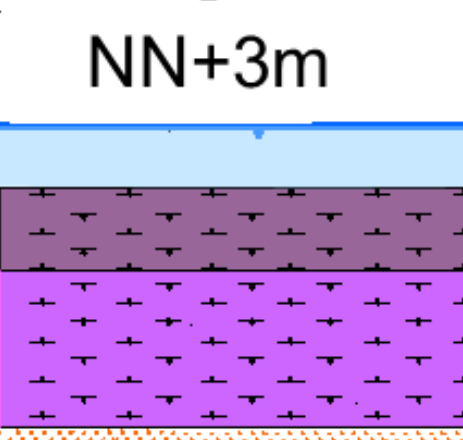
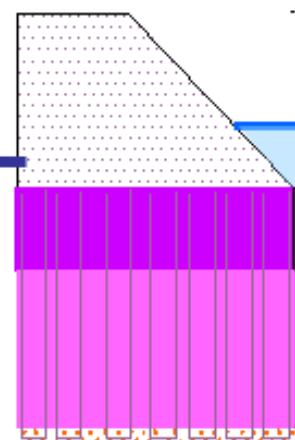
Hydraulic fill

Vertical drain

Horizontal drain

Dewatering

Fill to final elevation + estimated settlement or vacuum + fill



Very soft clay

Clay / peat

sand



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**Existing
Airbus Factory**



Reclaimed area

Closing dike





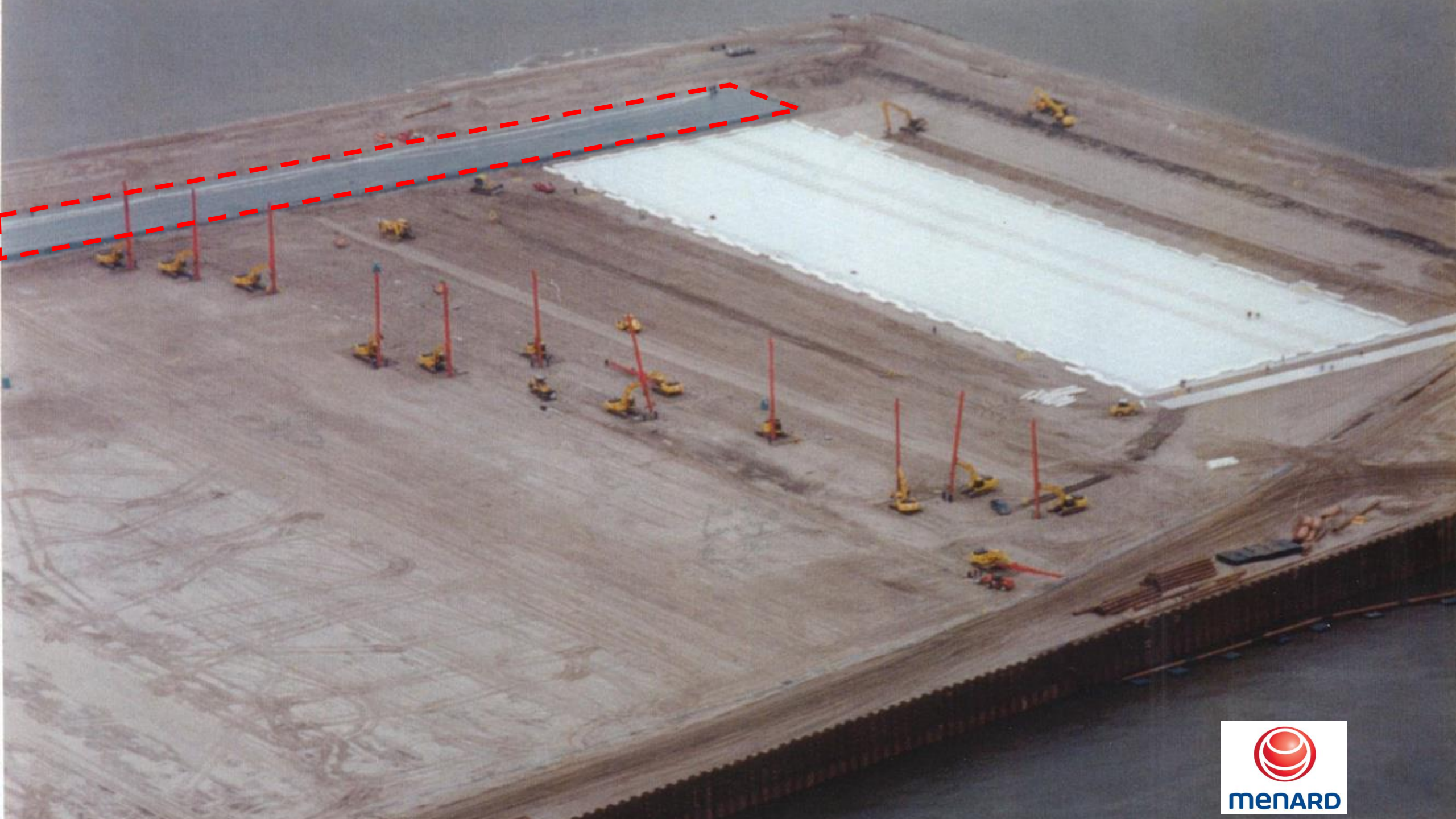










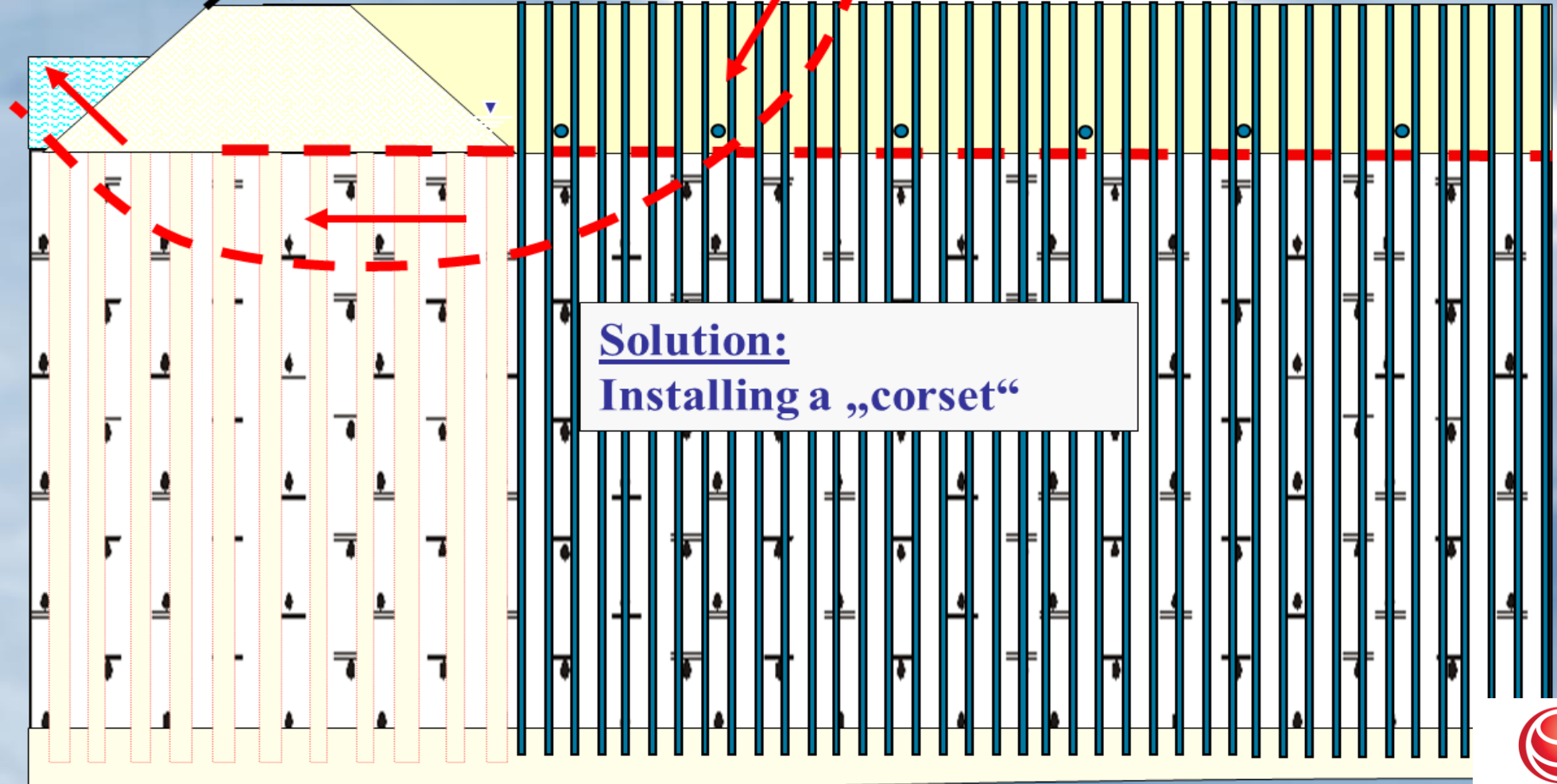


•_∇ +6,5 mNN

Stability problem

$\eta \ll 1,3$

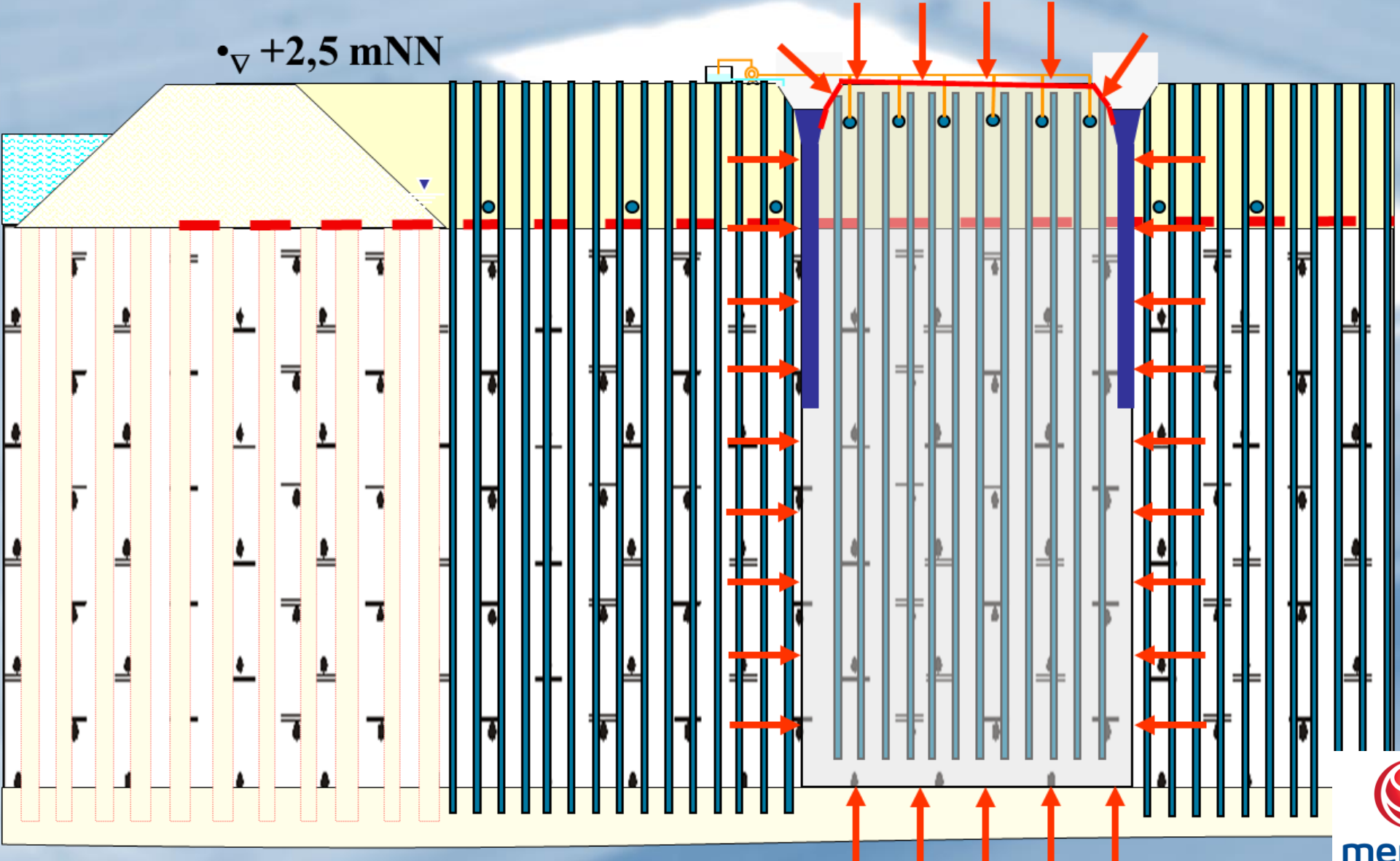
•_∇ +2,5 mNN

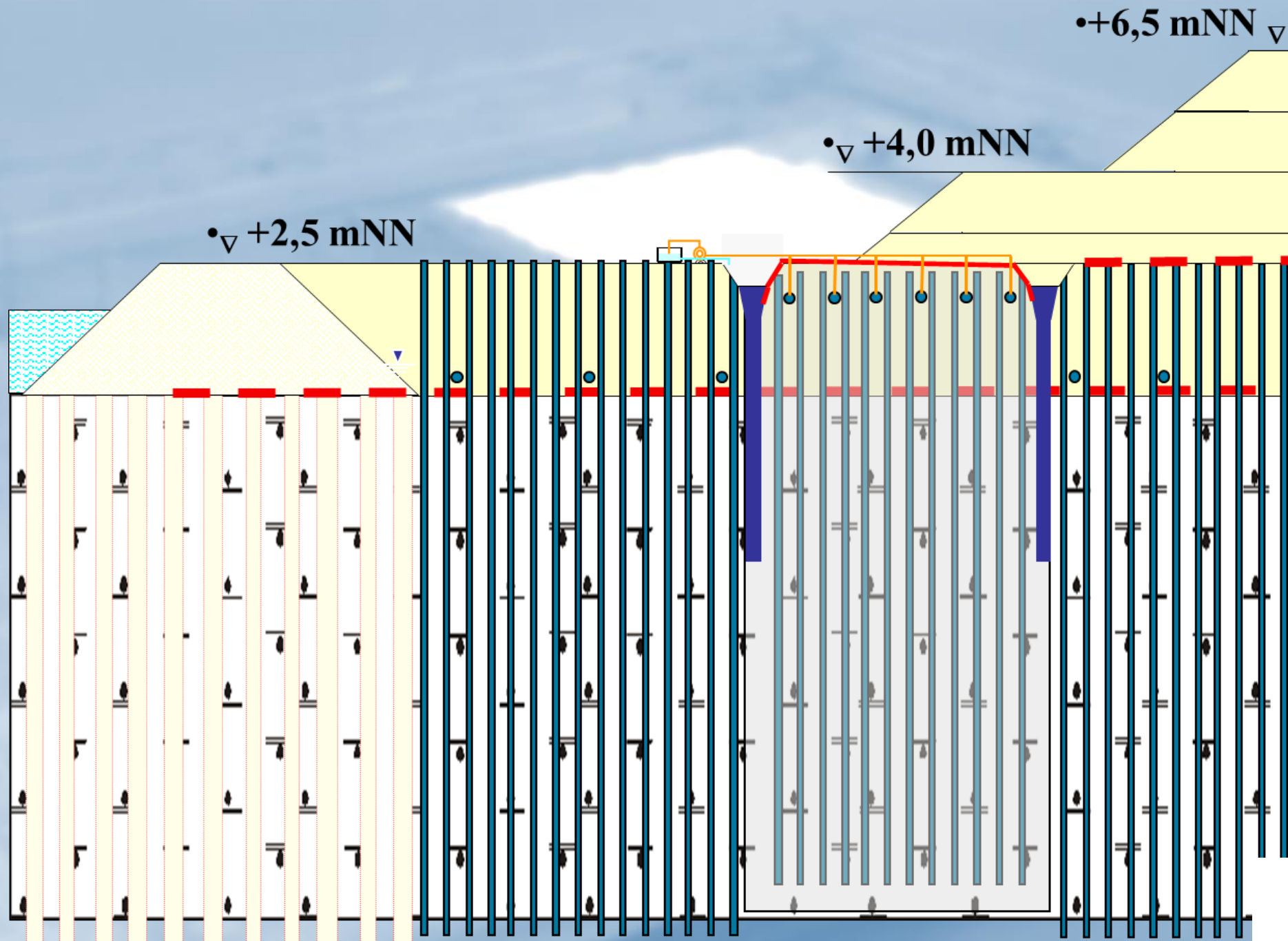


Solution:
Installing a „corset“

• Vacuum 70 kN/m² (0,7 bar)

• $\nabla +2,5$ mNN









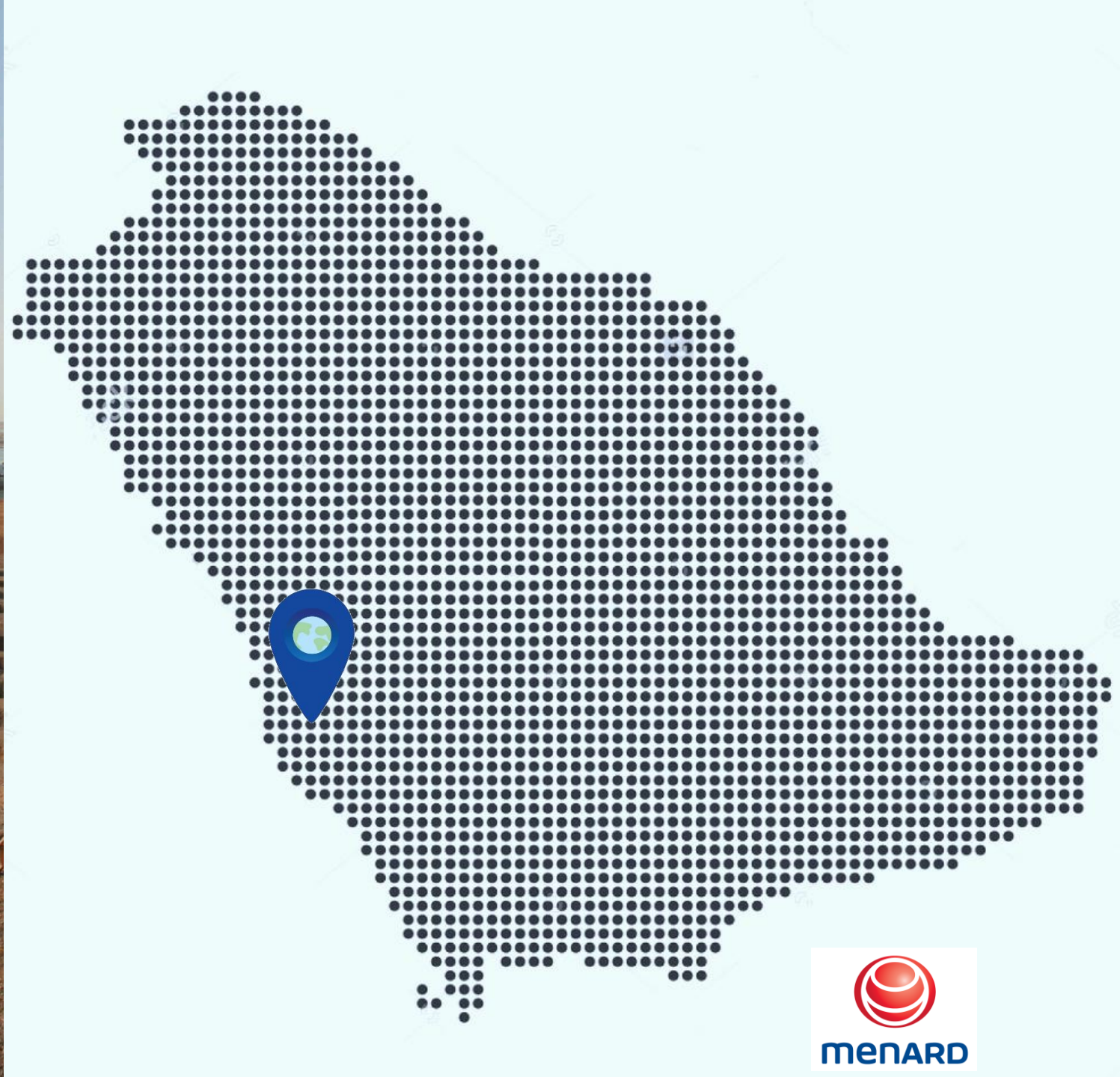


Project : King Abdulla University of
Science and Technology (KAUST)
Country : Saudi Arabia (Jeddah)
Summary : New University (3M m2)
built from scratch in record time in the
desert

Main Issues to Solve :

- Local condition : relatively
heterogeneous deposits of Sabkah (
loose silt deposited by wind)
- Fast track project and project not well
defined at time of ground
improvement
- High water table







- KAUST = King Abdullah University for Science and Technology
- New university campus of 36 million m² (i.e. 6 km x 6 km) to be completed in 26 months
- Located in the desert near Jeddah
- Includes desalination plant, wind turbines, golf course, residences, services, campus, infrastructures



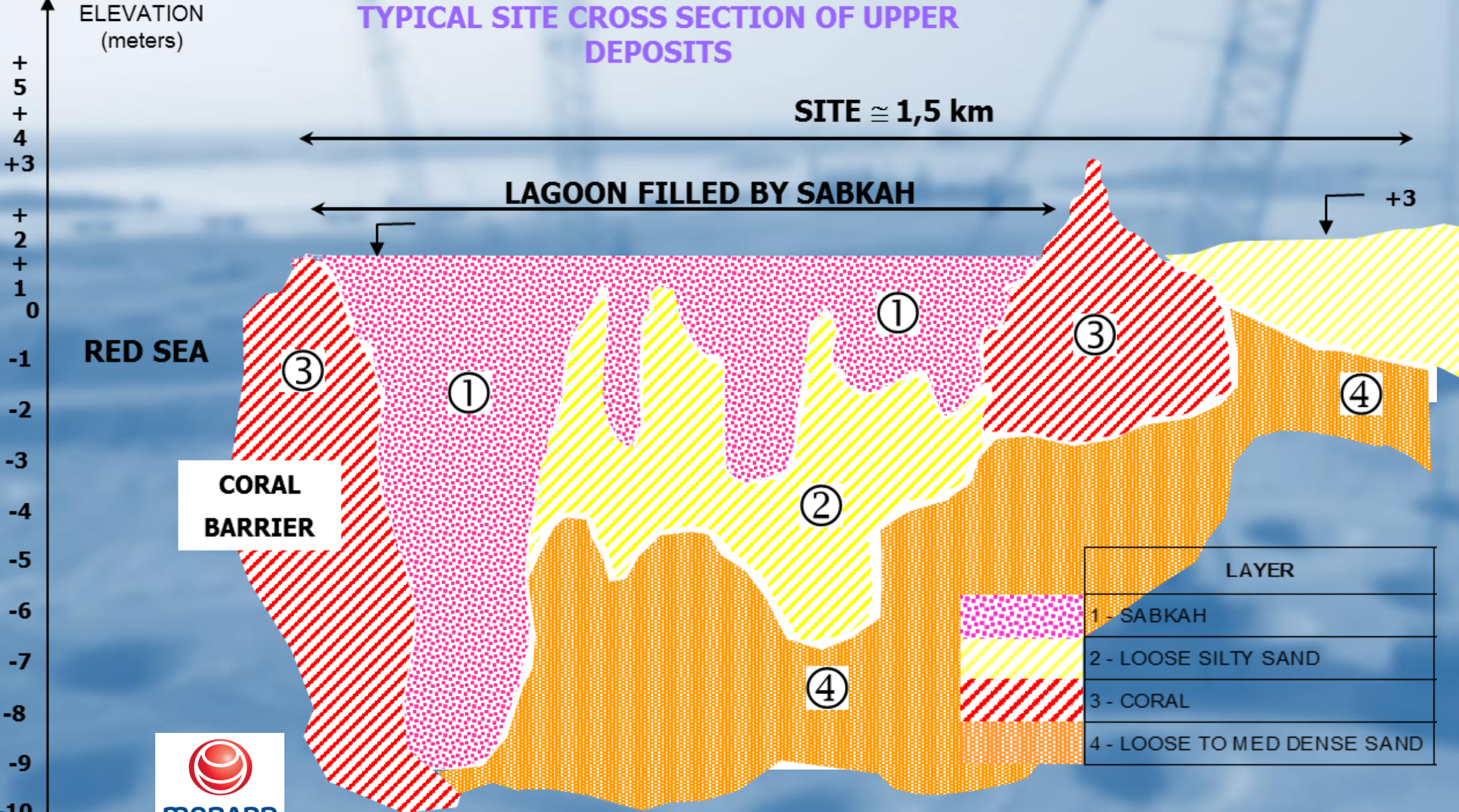
- Initial Conditions and Challenges :
 - Sabkah : saturated loose fine silty sand – wind blown . On this project, up to 5m thick at surface
 - Fast - track project : project was launched before 100% drawings – Menard needed to propose a ground improvement system without having final structural drawings and loads



TYPICAL SITE CROSS SECTION OF UPPER DEPOSITS

SITE \approx 1,5 km

LAGOON FILLED BY SABKAH



ELEVATION
(meters)

+5
+4
+3
+2
+1
0
-1
-2
-3
-4
-5
-6
-7
-8
-9
-10

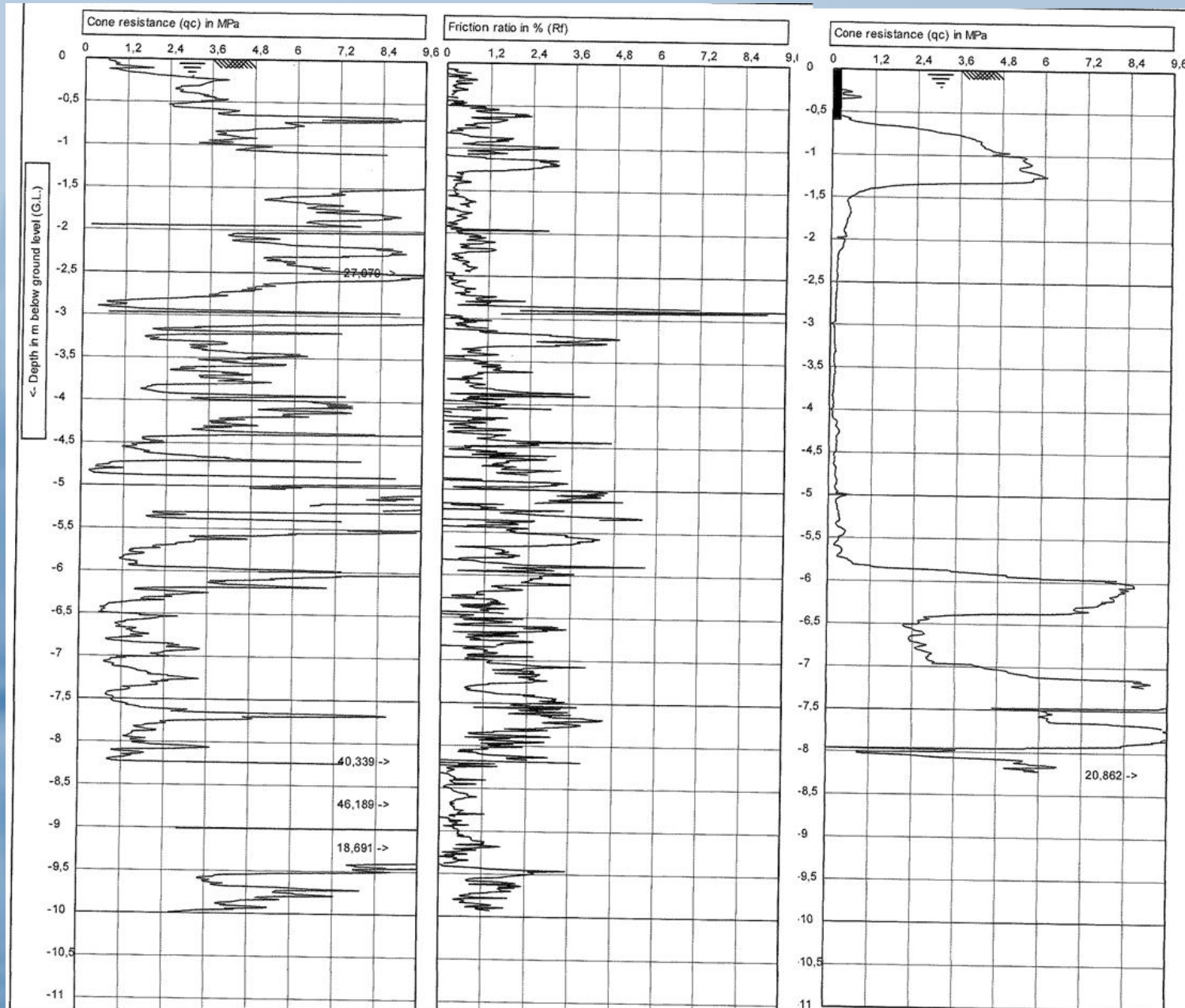
RED SEA

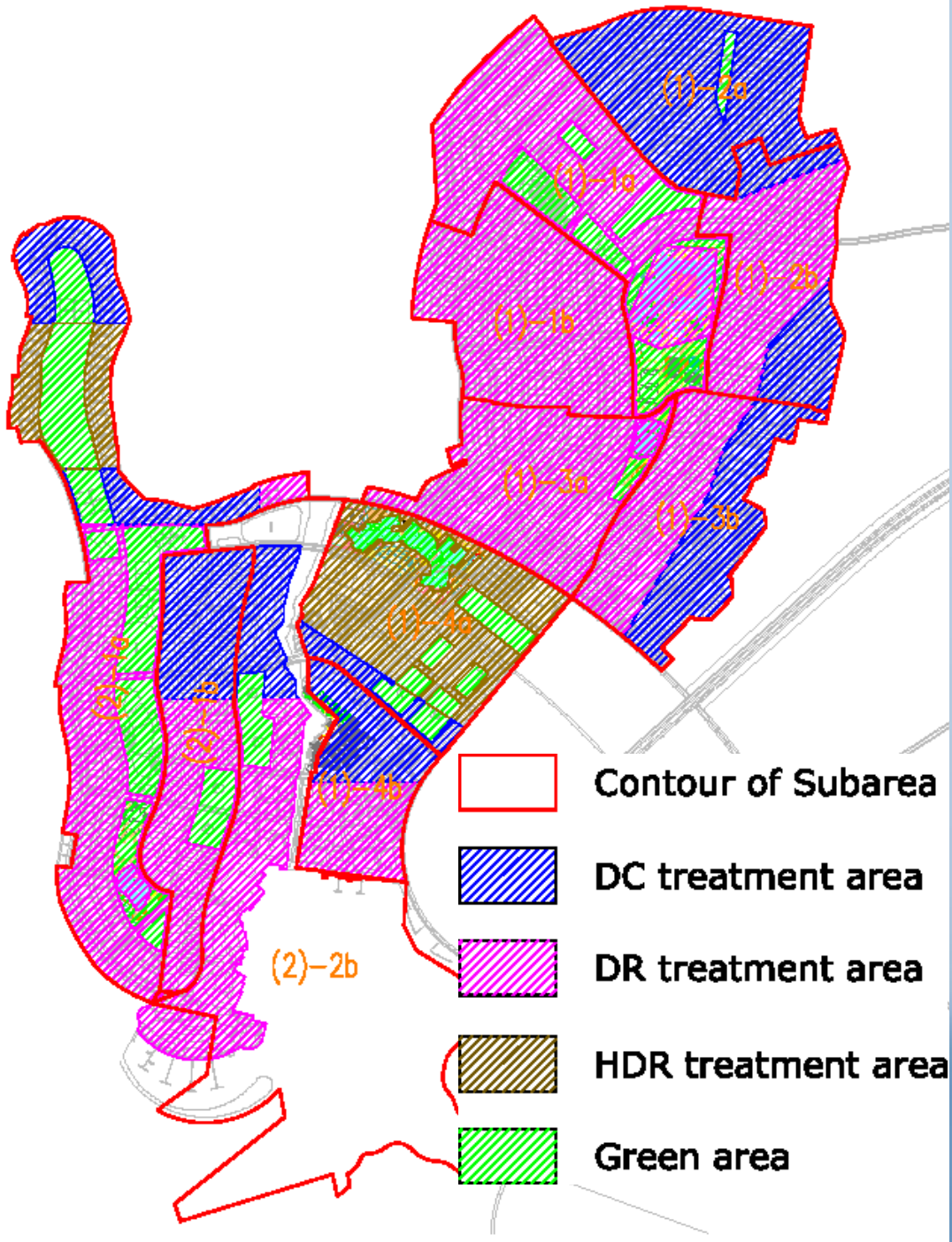
CORAL
BARRIER

| LAYER | |
|---|-----------------------------|
| | 1 - SABKAH |
| | 2 - LOOSE SILTY SAND |
| | 3 - CORAL |
| | 4 - LOOSE TO MED DENSE SAND |



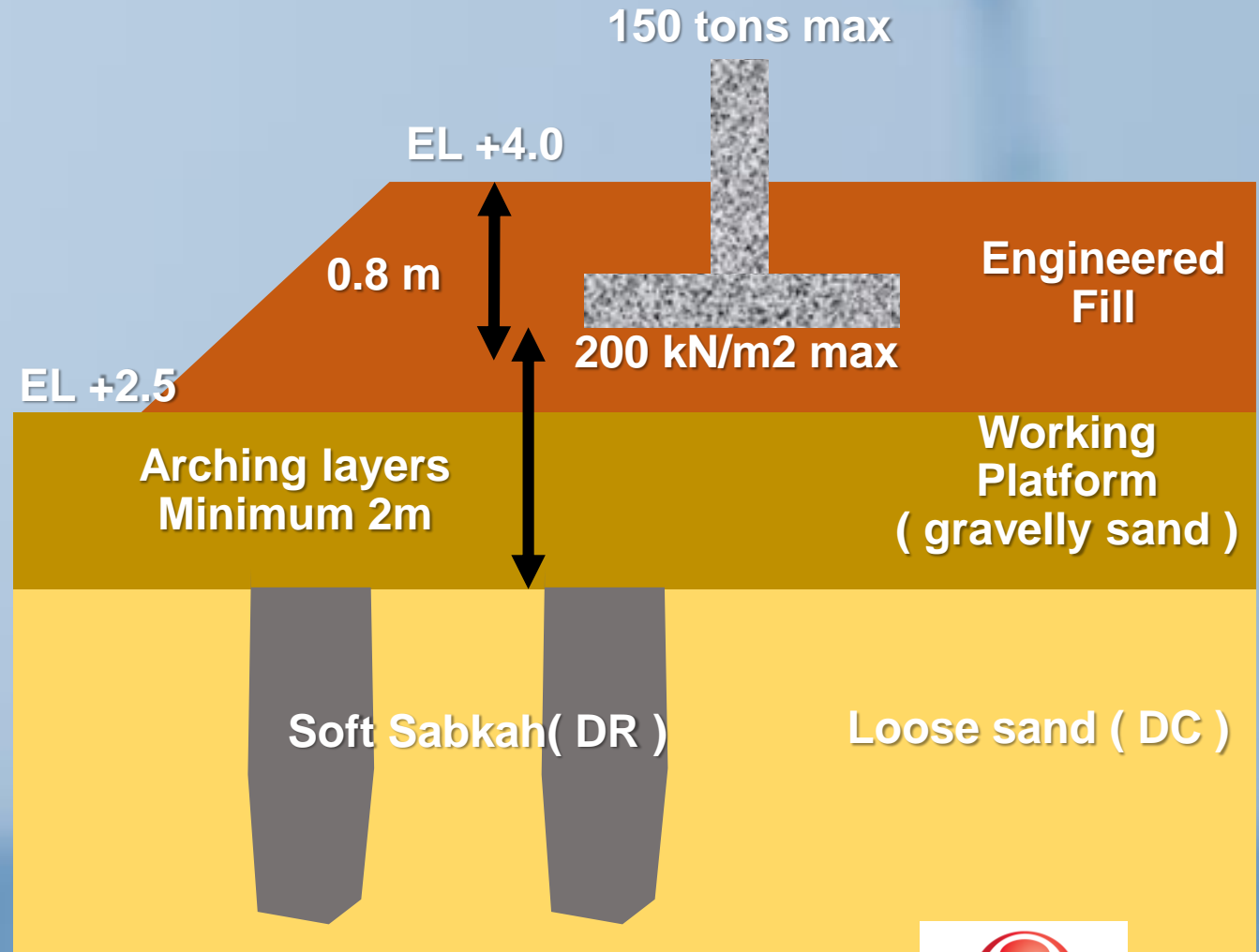
VARIATION IN SOIL PROFILE OVER 30 METERS

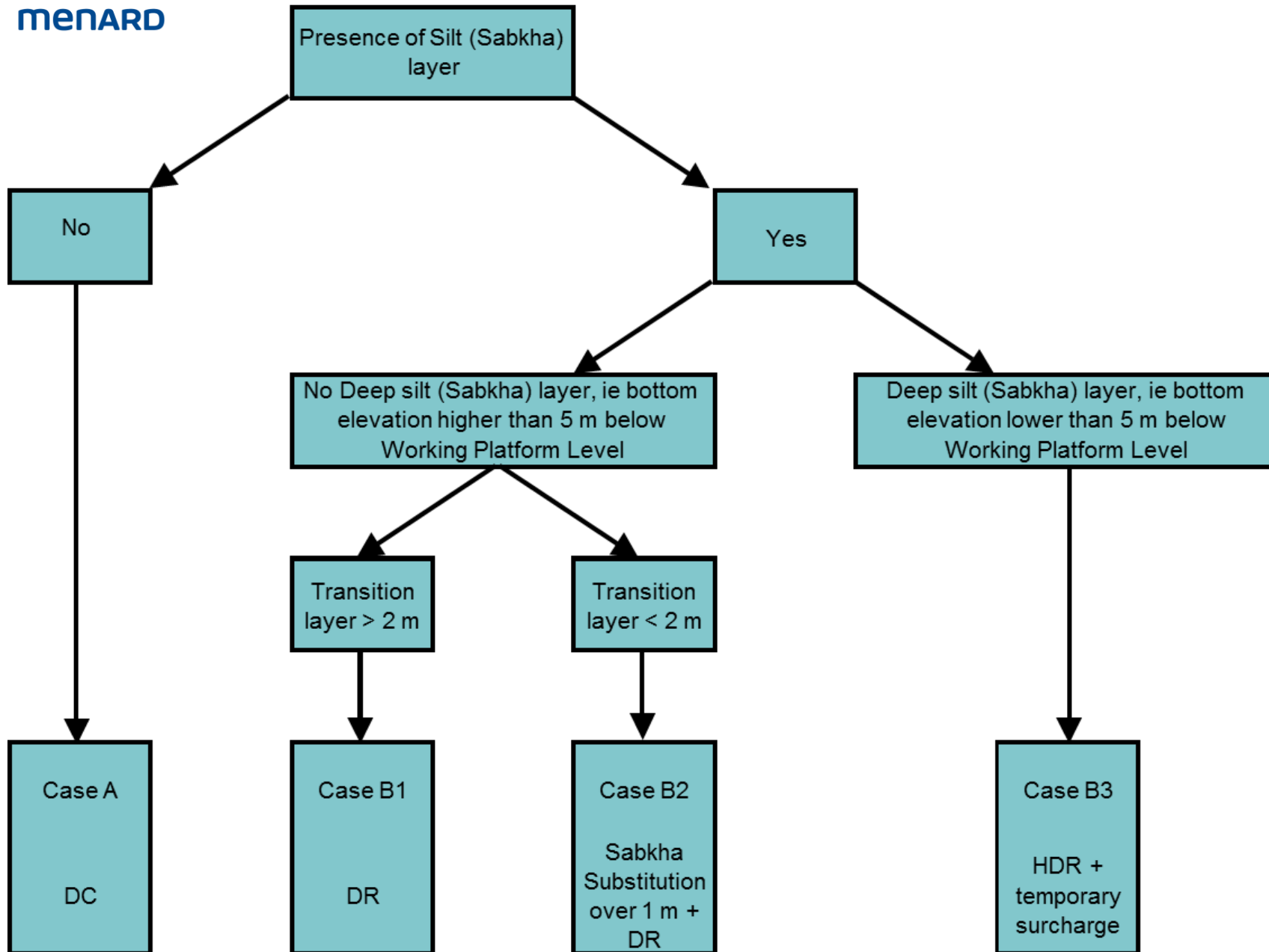




DESIGN CONCEPT WHEN LOCATION OF FOOTINGS UNKNOWN

Total settlement < 1 inch (25mm) – max diff 1/500 on footings





DESIGN DECISION TREE

Based on
Observational
Method

Selection of G.I.
method is
dependent on site
observation during
compaction and
borings



12 Cranes (LRB 855 & 885) x 2 shifts

12 to 25 tons weight depending on areas

Over 2,500,000 m² (25,000,000 ft²) of DC/DR

Team of 100 persons on site













Project : Fedex Ground Sorting Facility

Country : USA (Jersey City)

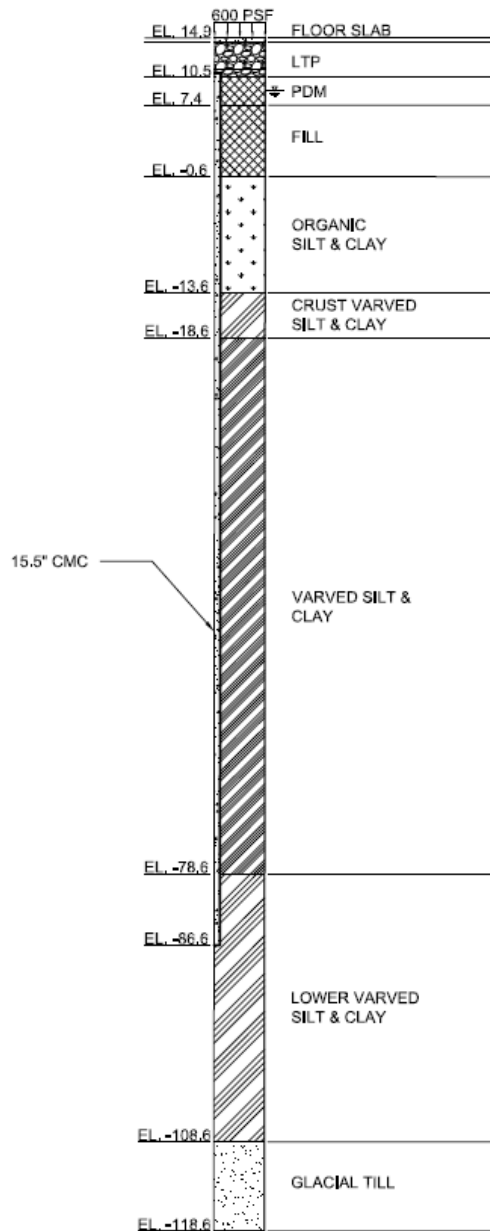
Summary : New plant in Greenfield with final grade raised several meters and large net new loads

Main Issues to Solve :

- Local condition : thick deposits of varved soft clay below a layer of thick organics
- Settlement : predicted long term settlement >2ft over 20 years due to deep soft clays
- Construction period – Fast schedule
- Limited budget







CONSTRUCTION STAGING:

1. INITIAL STAGE, EXISTING GROUND SURFACE EL. 11.5
2. EXCAVATE TO EL. 10.5, PLACE WORKING PLATFORM TO EL. 11.5, INSTALL CMC, AND FILL WITH LTP MATERIAL TO EL. 14.4
3. CONSOLIDATE
4. PLACE FLOOR SLAB TO EL. 14.92
5. APPLY 600 PSF FLOOR SLAB LOAD
6. CONSOLIDATE

Soil profile :

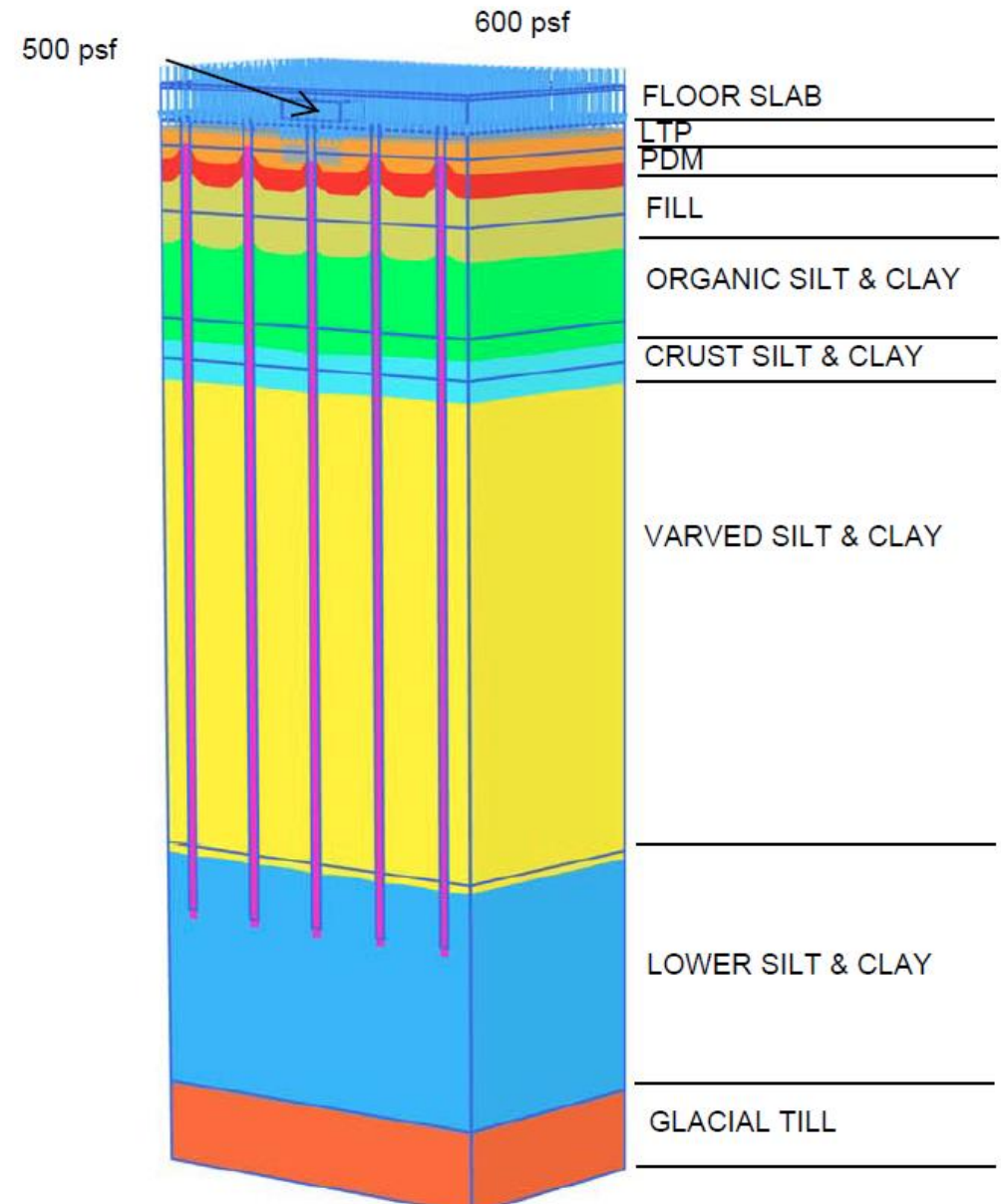
Fill (with some MSW) over Organics (meadow mat) over varved silt and clays

Main Challenge was thickness of compressible soil extending beyond the capacity of classical CMC rigid inclusions elements.

Second challenge was to limit total settlements to under 2 inches long term and differential settlement of $\frac{3}{4}$ inch between two column footings.

Differential settlement between loaded bay and unloaded bay is also to be studied

Differential between footings and slab is another focus





Depth challenge solved



Pull-down (t)

2

0

Torque (t.m)

2

4

Speed (m/mn)

2

0

Develop two custom made lead mast systems attached to crawler cranes with high torque / high pull down capacity



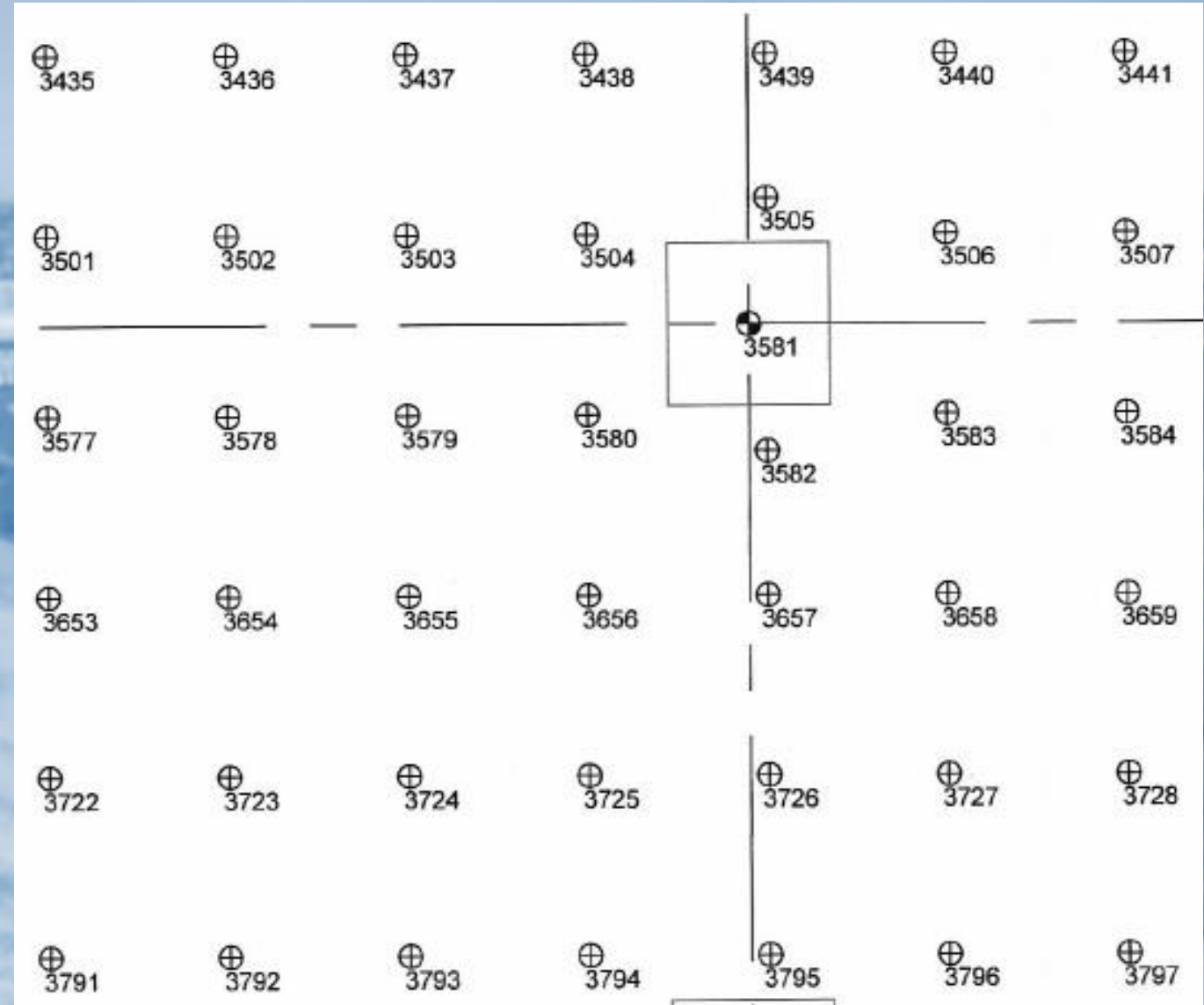
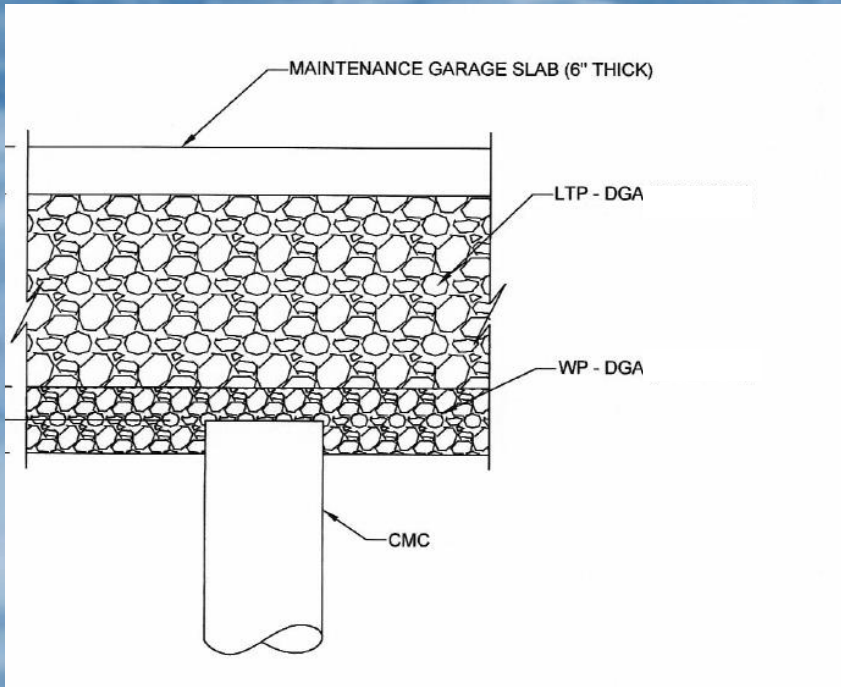
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Settlement challenge solve



Combination of Global support (with one added CMC at concentrated loads) and thick LTP to spread load





Fedex NJ

- ✓ Jersey City, NJ
- ✓ warehouse
- ✓ 350,000 sf
- ✓ 600 psf floor load
- ✓ 135 ft max
- ✓ 4,150 CMC











LESSONS LEARNED :

- Each site is unique and has its own challenges that lead to unique design-build solutions
- Innovate to find the right solution
- Being entrepreneurial and a risk-taker often pays off

THANKS !